EPA WORK ASSIGNMENT No.: 0762JZZ EPA CONTRACT No.: 68-W8-0110 EBASCO SERVICES INCORPORATED

ARCS II PROGRAM

DRAFT
SITE INSPECTION PRIORITIZATION REPORT
BROCKWAY MOTOR TRUCK SITE
CORTLAND
CORTLAND
CORTLAND COUNTY, NEW YORK
CERCLIS No.: NYD980203111
JULY 1995
VOLUME II OF III

NOTICE

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Cortland County Health Department Division of Environmental Health 60 Central Ave. P.O. Box 5590 Cortland, N.Y. 13045 607-753-5035

10 March 1995

Jeff Martin Ebasco Environmental Suite 435 2111 Wilson Blvd. Arlington, VA 22201-3001

Re: Ground Water Information, Site in Cortland, New York

Mr. Martin:

I have attempted to provide the information that you requested herein. Please note that the information is incomplete due to the age of some of the wells.

Cortland Water Board--3.5 MGD, 20,000 served, 5000 connections

<u>Well</u>	<u>depth</u> (<u>ft</u>)	feet of screen	<u>latitude</u>	longitude	% <u>use</u>
Well #3	77	32	42053 ' 42"	76 ⁰ 11'53"	80
Well #4	77	52	42053 ' 47"	76 ⁰ 11'49"	10
Well #5	54	20	42 ⁰ 35 ' 48"	76°11'43"	10

Cortland City Water is the only water supply that has experienced any incidence of contamination in the past, but the sample results never contravened water supply standards. The contamination was the result of a trichloroethene spill that occurred at the Smith Corona Typewriter Plant, although blame was never admitted. An air stripper was installed at the plant in anticipation of the plume.

Cortlandville Water Dept. -- 0.6 MGD, 4000 served, 1200 connections

<u>Well</u>	<u>depth</u> (<u>ft</u>)	feet of screen	latitude	longitude	% <u>use</u>
Well #1	65		42034 ' 41 "	76 ⁰ 12 ' 40"	13
Well #2	65		42034 41"	76012'41"	12
Well #3	64		42034'42"	76 ⁰ 12'40"	25
Lime Hollow	92	22	Lime Ho	llow Rd.	50

Newton Water Works--Village of Homer--0.9 MGD, 4400 served, 1248 connections

<u>Well</u>	<u>depth</u> (<u>ft</u>)	<u>feet of screen</u>	<u>latitude</u>	longitude	%use
Well #2	75		42038135"	76 ⁰ 11'25"	0
Well #3	83	30	42 ⁰ 38 ' 33"	76 ⁰ 11'27"	100

Village of McGraw--0.1 MGD, 1300 served, 412 services

<u>Well</u>	<u>depth</u> (<u>ft</u>)	<u>feet</u> of <u>screen</u>	latitude	longitude	% <u>use</u>
Academy #1	138		42035136"	76°05'48"	50
Academy #2	143	12	42035'36"	76 ⁰ 05 ' 48"	50
Bennett St.	140.5	12	42 ⁰ 35 ' 30"	76°06'11"	0

Polkville, the area between Cortland City and the Village of McGraw, has many individual and public water supply wells. The public water supply wells consist mainly of restaurants and motels.

Blodgett Mills is also a small hamlet also in the survey area. It is served by individual water supply wells.

I have highlighted these important areas on the map. Please contact this office if we can be of any more assistance.

Sincerely,

Peter W. Rynkiewicz

Asst. Public Health Engineer

Enclosure



Notification of Hazardous Waste Site

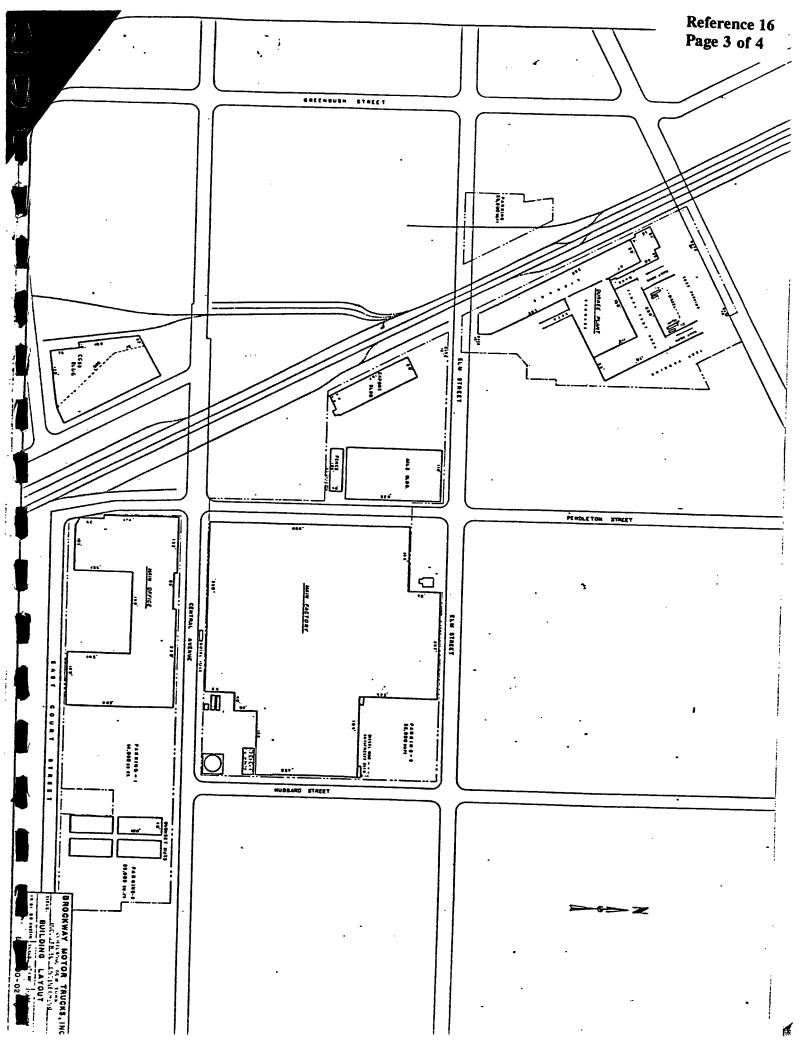
Reference 16 Page 1 of 4

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, initial notification information is duired by Section 103(c) of the Compresensive Environmental Response, Compensation, and Liability Act of 1980 and must be mailed by June 9, 1981.	paper indicate the letter of the see	 000 001 394

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Ā	Person Required to Notify: Enter the name and address of the person	Name Mack T	rucks, Inc.				
1	or organization required to notify.			d Book Offi			
				d, Post Offic	e Box	M	
 		Cav Allent	own	State	PA	Zip Code	18105
8	Site Location:			•			
	Enter the common name (if known) and actual location of the site.	Name of Sae Brockw	ay Motor Tru	cks (inactive	: opera	tion an	d site)
Î	deliber location or the site.	Sireer 106 Ce	ntral Avenue	1			
)	NYD 980203111	Cov Cortla		ortland State	NY		120/5
ō	Person to Contact:			31919		Zip Code	13045
	Enter the name, title (if applicable), and	Name (Last, First and Tak	n Mack T	rucks, IncI	egal D	epartme	nt
	business telephone number of the person to contact regarding information	Phone (215) 43	9-3116				
ì	Submitted on this form.	(223) 43	<i>y</i> -3110	•	• -		
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ð	Dates of Waste Handling:						"
ì	Enter the years that you estimate waste treatment, storage, or disposal began an	e From (Year) 1969	To (Year)	1977			
į.	ended at the site.		10 (1487)				
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}	Mana Turn Channel						
F	Waste Type: Choose the option you	•					
_	Option I: Select general waste types and you do not know the general waste type:	source categories. If	Option 2: Th	is option is availa	ble to per	sons fam	iliar with tr
6	encouraged to describe the site in Item I	-Description of Site.	LESOUICE CO	nservation and Re 40 CFR Part 2611	covery A	t (RCRA)	Section 30
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Form Approved OMB No. 2000-0138

on of mezardous Waste Site	Size Two		Reference 16
. Juantity	Facility Type	Total Facility Wa	Page 2 of 4
In X in the appropriate boxes to lite the facility types found at the site	1 D Piles 2 D Land Treatment	Cubic feet	
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, we the estimated combined quantity (volume) of hazardous wastes at the site	4 Tanks	Total Facility Area	
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Occupy using square feet or acres	. 8 Drums, Below Ground 9 Dother (Specify)		
Known, Suspected or Likely Releases t			
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Note: Items Hand I are optional. Completing hazardous waste sites. Although completing	I these items will assist EPA and State g the items is not required, you are en	e and local governments in acouraged to do so.	locating and assessin
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Sketch a map showing streets, highways, routes or other prominent landmarks near	SEE ATTACHED BUI (DRAWING BB-00-0		•
the site. Place an X on the map to indicate the site location. Draw an arrow showing			
the direction north. You may substitute a publishing map showing the site location.	•		
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Description of Site: (Optional)	74	ine to brockway plant.	
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Industrial wastes were deposited on the site, which has since be	i from time to time an covered by new		
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Signature and Title:			
The person or authorized representative (such as plant managers, superintendents.	Name Kenneth A. Blythe,	Staff Attorney	Owner, Present
trustees or attorneys) of persons required to notify must sign the form and provide a	Mack Trucks, Inc. Sweet 2100 Mack Boulevard	. P. O. Box M	Owner, Past Transporter
mailing address (if different than address			Operator, Present
in item A). For other persons providing notification, the signature is optional	Cev Allentown, State	PA Ze Code 18105	Operator, Past
Check the boxes which best describe the relationship to the site of the person	Neweth a Cathe	o June 8, 19	Other 81
required to notify If you are not required	Signature Vittamin Control	- ONE DUILE OF XX	



UNITED STATES

ENVIRONMENTAL PROTECTION AGENCY

REGION III

OFFICIAL BUSINESS

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SHY ROUMENTAL PROTECTION ASSESS

EPA-335



U.S. SPA REGION 2 SIZES NOTIFICATION NEW YORK, NY 1007

17



December 11, 1986

Mr. William Lewis Rubbermaid 106 Central Avenue Cortland, NY 13045

Reference: Hydrocarbo: Investigation

Dear Mr. Lewis:

This letter report presents the findings and conclusions from our hydrocarbon investigation at your facility in Cortland, New York. Our investigation was authorized on November 10, 1986 under P. O. No. 10277, pursuant to our proposals of October 27 and November 7, 1986. This report has been prepared for the exclusive use of Canford Manufacturing Corporation with specific application to your facility in Cortland, New York. Our work was performed using generally accepted hydrogeologic practices. No other warranty, expressed or implied, is made.

Project Background

A buried 1000 gallon tank was uncovered in September, 1986 during excavation for a dry well. After the tank was removed, samples of water and sludge in the pit were taken and analyzed for benzene, toluene and xylene. The results showed 49 ug/l benzene, <1 ug/l toluene and 1760 ug/l xylene in the water sample, and no detectable concentrations of benzene, toluene or xylene in the sludge sample (verbal communication, William Lewis, October 23, 1986). Based on these results, further analysis of the water sample was performed for volatile halogenated compounds (EPA 601) and hydrocarbon_identification (DOH 310-13) (see Appendix C). No halogenated compounds were found. The laboratory tentatively identified the hydrocarbon as gasoline, but indicated the pattern did not match gasoline exactly. The laboratory also indicated the benzene previously reported was not present.



Mr. William Lewis Rubbermaid Page 2 December 11, 1986

Based on these results, Rubbermaid was requested by the NYSDEC to install 3 monitoring wells, obtain water samples from the wells and determine the direction of ground water flow. This report presents the findings and conclusions from this investigation.

Methodology

Three wells were installed in the vicinity of the tank (removed before our investigation). The location of the wells is shown on Drawing No. 1, Appendix A. One well (B-1) was placed in a presumed upgradient location and two wells (B-2 and B-3) were placed in presumed downgradient locations. The borings for the wells were advanced using 4 irch diameter flush joint casing. Soil samples were taken during drilling using a split spoon sampler to investigate subsurface conditions. Continuous soil samples were taken from a depth of 4 to 5 feet to a depth of 14 to 16 feet (see boring logs, Appendix B for depth of soil samples). Continuous soil samples were taken in this interval to identify whether a reported confining clay unit was present.

Monitoring wells were installed in the borings using two inch diameter flush joint, threaded. PVC riser pipe and well screen. Clean silica sand was placed around the well screen and a seal of bentonite pellets was placed above the sand pack. The wells were finished by placing a locking cap on the well and cementing a curb box flush with the ground surface over the well. Well construction details are shown on the boring logs in Appendix B.

The wells were developed by surging and pumping the wells until the water was clear. Water samples were taken on November 25, 1986 by Empire-Thomsen and delivered to Galson Technical Services for analysis. Before taking each water sample, the wells were purged using a pump until the conductivity of the water stabilized (<10% change between measurements). Twenty gallons were purged from each well to reach a stabilized conductivity. Water samples were taken using a PVC bailer. All sampling equipment was washed with soap and water and rinsed with clean tap water and distilled water between wells. Clean tubing was used to purge each well. A blank was prepared following water sampling using tap water from the facility.



Mr. William Lewis Rubbermaid Page 3 December 11, 1986

The location and elevation of the wells were surveyed on November 25, 1986. Elevations were determined using differential levelling and referenced to an assumed elevation of 100.00 for the east corner of the doorway to the metal storage building (see Drawing No. 1 for benchmark location). The well locations were taped from the metal storage building. Water levels were also measured on November 25, 1986 to determine the direction of ground water flow.

Findings and Conclusions

The three borings encountered 3 to 5-1/2 feet of fill below the ground surface. In boring B-1 and B-3 an organic silt unit was found beneath the fill. In boring B-2 medium to fine sand was encountered below the fill. All three borings encountered gravel and sand between a depth of 6 to 6.5 feet and the bottom of the boring (22 to 26 feet). The water table in the three borings ranged from 9 feet below ground surface in B-1 to 8.5 feet below ground surface in B-2. The water table in the borings is within the sand and gravel aquifer.

The water levels in the wells were used to determine the direction of groundwater flow (see Drawing No. 1, Appendix A). The direction of groundwater flow is toward the east, southeast, which conforms to the regional direction of groundwater flow in the aquifer. Wells B-2 and B-3 are downgradient of the location of the removed tank while B-1 is upgradient of the removed tank.

No detectable hydrocarbons were found in the three well water samples (see Appendix C). Although hydrocarbons were found in the water directly below the tank when it was removed, hydrocarbons are not currently found in the aquifer downgradient of the tank location. The absence of hydrocarbons in the aquifer downgradient of the removed tank is probably the result of one or more of the following factors: 1) removal of the tank, 2) attenuation of hydrocarbons in the organic silt unit before the hydrocarbons reach the underlying aquifer (the silt unit was found in borings B-1 and B-3 and also observed below the tank), 3) attenuation of hydrocarbons in the sand and gravel aquifer due to high flow velocities in the aquifer.



Mr. William Lewis Rubbermaid Page 4 December 11, 1986

Since no contamination was found in the wells, we do not think any further monitoring of the site is needed.

Respectfully submitted,

EMPIRE-THOMSEN

May 73. Rundalo - Lee

Marjory Rinaldo-Lee, C.P.G.S.

MRL:er cc: File

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1	1	ELEVAT	Z			10	14	2" Crushed Stone; 8" Concrete FILL: Brown/Grey SILT, SAND & GRAVEL w/Cinders (Noist) 5.5' TOPSOIL (Organic Silt) .G.0' Brown GRAVEL, little sand & silt (Wet-Compact)		5M30	WATER CONTENT	(cm/)**Ci		11-25					Cand Iµmha cm1	Eh ImVI	ρΗ	MELL CONSTRUCTION Screen: 2" Ø PVC, 0.002" slots, 15' long, 21.7'-6.7' Riser: 2" Ø PVC, thraded, flush joints Sand Filter: 4 Q silica Sand,
	21.						35	- · · · · · · · · · · · · · · · · · · ·														Bentonite Seal: 6'-4' Metal curb box cemented over top of well flush with ground surface, locking cap installed on well At completion of drilling, water 9.05', casing at 19.0' Elevation at Top of Well=79.71
		sy and nation			NO NO.	PACCOVERY		Surface Elevation 79,90 Outs Starled 11-24-86 Date Completed 11-24-86 Number of Installations in Soring 1 Method of Installation 4" dia, Flush Joint Casing		Proj Loca	ect No. ect Tit		ald Cor		P/-1				T	HO	M	GEOLOGIC LOG SEN MONITOR NO. 8-3 Sheet 1 at 1

ı	ATION	LES	LE NO.	VERV	2	SOIL of ROCK CLASSIFICATION	ED SIF.	1.1	ENT	PERMEABILITY		MONITOR OUSTRUC	TION			WATE	PROB	Ε		
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See k	ay and nation to		RUN NO	RECOVERY [Percent]	300	Date Started 11-21-86 Date Completed 11-21-86 Number of Installations in Boring 1 Method of Installation 4" dia, Flush	_ '	oc #11	٥٥	Rubberma Corrland Hydrogeolo	New Y	ork		 		T	HQ	M.	EN	MONITOR NO.

	8	ı,	9 4 4 3				<u> </u>	¥ =	PERME ABILITY		TOR/PIEZO			WATER READ	PROBE		
DEPTH	ELEVAT	SAMPLE	SAMPLE CHEM S RECOVI	2	SOIL OF ROCK CLASSIFICATION	UNIFIED SOIL CLASSIF.	DENSIT	WATER CONTE	icm/seci		Water Level	I	Temp	Cond	En	ρН	NOTES
10-20-25		7	1 12 2 17 3 9 4 7 5 NR 6 9	12	Fill: Grey GRAVEL & SAND, trace Silt (Dry) 4.5' Grey SILT & CLAY with organics, trace Sand (Moist-Hedium) 6.5' Brown SAND, Some Gravel & Silt (Met-Firm) Brown GRAVEL, Some Sand, little silt (Met-Firm) Boring Terminated @ 23.5'						11-25		1*C1	ijimha em	Imvi		MELL CONSTRUCTION Screen: 2" Ø PVC, .002" slots 15" Long. 22'-7' Riser: 2" Ø PVC, threaded flush joints Sand Filter: 4 Ø silica Sand 23.5'-6.5' Benonite Seal: 6.5'-4.5' Metal Curb Box cemented over well flush with ground surface, locking cap installed on well At completion of drilling, wite at 9.8', casing at 22.0' Elevation Top of Well*29 09
	E: ley and ination to		RUM NO. RECOVERY	goa	Surface Elevation 100.15 Data Started 11-20-86 Data Completed 11-21-86 Number of installations in Boring 1 Method of installation 4" dia, Flush 3		Local	on	Rubbermaid Cortland,	Corporation				I	10l	VIS	EDLOGIC LOG Reference to the second

Reference 17 Page 8 of 8

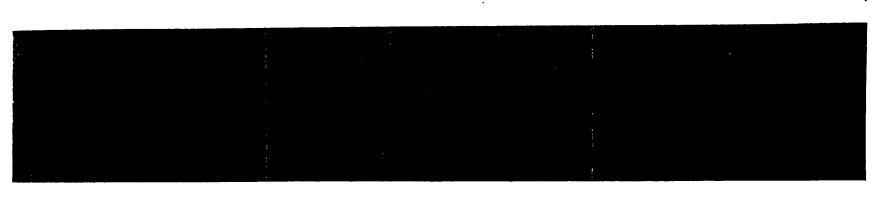
TECHNICAL PAPER NO. 40

RAINFALL FREQUENCY ATLAS OF THE UNITED STATES

for Durations from 30 Minutes to 24 Hours and Return Periods from 1 to 100 Years

Prepared by
DAVID M. HEUSIFIELD
Comperative Studies Section, Hydrologic Services Division
for
Engineering Division, Soil Conservation Service
11.8, Department of Agriculture





Reference 19 Page 1 of 3

Series 1957, No. 10

SOIL SURVEY

Corlighd County New York



UNITED STATES DEPARTMENT OF AGRICULTURE

Soil Conservation Service

In cooperation with

CORNELL UNIVERSITY AGRICULTURAL EXPERIMENT STATION



Palmyra Series

These medium-textured soils have an accumulation of clay beginning at depths between 12 and 18 inches. Even though the surface soil is medium to slightly acid, free lime commonly begins at depths between 26 and 40 inches. The soils are mainly on nearly level to gently sloping outwash plains in the valleys north of the city of Cortland and near Cuyler. They have formed in deep, gravelly outwash derived from siltstone, sandstone, shale, and darkgray limestone. Because the underlying outwash material is moderately to rapidly permeable to water, the soils are well drained.

The Palmyra soils belong to the Gray-Brown Podzolic great soil group. Most commonly they are associated with moderately well drained Phelps and poorly drained

Typical profile (Palmyra gravelly silt loam, 0 to 3 percent slopes; cultivated):

 A_{P} 0 to 8 inches, dark grayish-brown (10YR 4/2) gravelly silt loam; weak, fine to medium, crumb structure; friable; fine roots abundant; pH 6.6, limed; abrupt,

smooth lower boundary.

8 to 12 inches, brown (10YR 5/3) to yellowish-brown (10YR 5/4) gravelly silt loam; compound structureweak, fine, crumb and very weak, thin, platy; friable; fine roots abundant; pH 5.7; 2 to 5 inches thick; clear, smooth lower boundary.

Bn 12 to 18 inches, dark-brown (10YR 4/3) to dark yellowish-brown (10YR 4/4) gravelly silt loam; weak, fine, blocky structure; friable, slightly hard; fine to medium-sized roots plentiful; pH 6.4; 5 to 12 inches thick; clear, wavy lower boundary.

B₂₂ 18 to 29 inches, dark-brown (10YR 4/3) to very dark grayish-brown (10YR 3/2) gravelly clay loam with many very dark gray (N 3/) and dark reddish-brown (5YR 3/2) pebbles of weathered limestone; weak to moderate, fine, subangular blocky structure; friable when moist, slightly sticky when wet, slightly hard when dry; firmer in place than horizons just above or below; a few medium-sized to large roots; contains a few cobblestones and other stones; pH 6.8; 8 to 18 inches thick; clear, irregular lower boundary.

C₁ 29 to 40 inches, dark grayish-brown (10YR 4/2) very gravelly loam or sandy loam that has fewer pebbles of weathered limestone than material in the horizon just above; essentially structureless; loose; a few large and medium-sized roots; weakly calcareous; cobblestones and other stones are common, and many of them have a thin coating of carbonates: 8 to 20 inches thick; gradual, wavy lower boundary

C₂ 40 to 60 inches +, dark grayish-brown (10YR 4/2) gravelly, cobbly, and stony loam to sandy loam outwash material; structureless; loose; a few large

roots; calcareous.

In some places near Preble, carbonates are at a depth of only 16 inches in these soils. They begin at depths of as much as 40 inches in places near Cortland where the Palmyra soils grade to the Howard soils. The thickness of the clayey B₂₂ horizon varies: In many places tonguelike projections extend downward from the B₂₂ horizon into the C₁ horizon; in other places, just a few feet away, the B₂₂ horizon begins a little below the plow (A_p) layer. In places the C₂ horizon is several feet thick.

These soils are permeable to air and water, and roots penetrate easily. The gravelly silt loams do not contain enough gravel to interfere seriously with cultivation. The cobbly loam, however, contains enough cobblestones

to make cultivation difficult.

These are among the most productive and highly responsive soils in the county. They have medium texture and a high content of lime. When fertilized with potash and phosphate, all crops grown in the area are suitable. The crops most commonly grown are corn for silage and grain, oats, wheat, alfalfa, and grass-legume hay crops and pasture. A small acreage is used to grow potatoes, dry beans, snap beans, cabbage, and peas.

The outwash deposits in which these soils formed are an important source of gravel for highway and building construction. Because the gravelly outwash is permeable to water, these soils are excellent sites for industrial

development.

Palmyra gravelly silt loam, 0 to 3 percent slopes (PbA).—A profile of this soil is described as typical of the series. A few acres of gravel-free silt loam were included in mapping. The soil that is free of gravel is more acid at depths between 26 and 30 inches than the typical soil. Most of it is in an area 11/2 miles northeast of Preble. Included with this soil are a few acres near Homer where the surface layer consists of gravelly sandy loam.

Palmyra gravelly silt loam, 0 to 3 percent slopes, is high in lime, has good moisture-holding capacity, and is easy to work. The soil absorbs water readily, and erosion

is not a problem.

This soil is suited to intensive cultivation. It can be used for all of the crops commonly grown in the county, and yields are high. For continuous high yields, the supply of available forms of nitrogen, phosphorus, and potassium should be supplemented to the extent necessary. This soil is in capability unit I-1.

Palmyra gravelly silt loam, 3 to 8 percent slopes (PbB).—This soil has a profile like that of the soil described as typical of the series, but it has stronger slopes. In a small acreage the relief is undulating and the slopes are short and broken. Included with this soil are a few acres near Homer where the surface layer consists of

gravelly sandy loam.

This productive soil is high in lime, absorbs water readily, and is easy to work. It is well suited to intensive cultivation and can be used for all the crops commonly grown in the county. The soil requires about the same general management as the less strongly sloping Palmyra soils. If used for continuous row crops, however, it needs protection from erosion. This soil is in capability unit IÎe-1.

Palmyra gravelly silt loam, 8 to 15 percent slopes (PbC).—This soil resembles the soil described for the series, but most of it has short, steep slopes that are complex.

In a small acreage it has simple slopes.

Runoff is rapid and the soil is somewhat droughty. The soil is more likely to erode than the less strongly sloping Palmyra soils and is not so easy to work. Nevertheless, it is productive and can be used for all the crops commonly grown in the county. Long-lived varieties of alfalfa, mixed with smooth bromegrass and grown for hay, are especially well suited to this Palmyra soil because of the ability of the alfalfa to extract moisture from depths of 3 to 4 feet in the soil profile. This soil is in capability unit IIIe-1.

Palmyra gravelly silt loam, 15 to 25 percent slopes (PbD).—This soil has a profile similar to that of the soil described as typical of the series. In general, however, the depth to the clayey horizon (B_{22} horizon in the typi-

Hydrogeology of the Surficial Outwash Aquifer at Cortland, Cortland County, New York



Prepared in cooperation with
SUSQUEHANNA RIVER BASIN COMMISSION



HYDROGEOLOGY OF THE SURFICIAL OUTWASH AQUIFER AT CORTLAND, CORTLAND COUNTY, NEW YORK

By Richard J. Reynolds

U.S. GEOLOGICAL SURVEY

Water-Resources Investigations Report 85-4090

Prepared in cooperation with the SUSQUEHANNA RIVER BASIN COMMISSION



Albany, New York

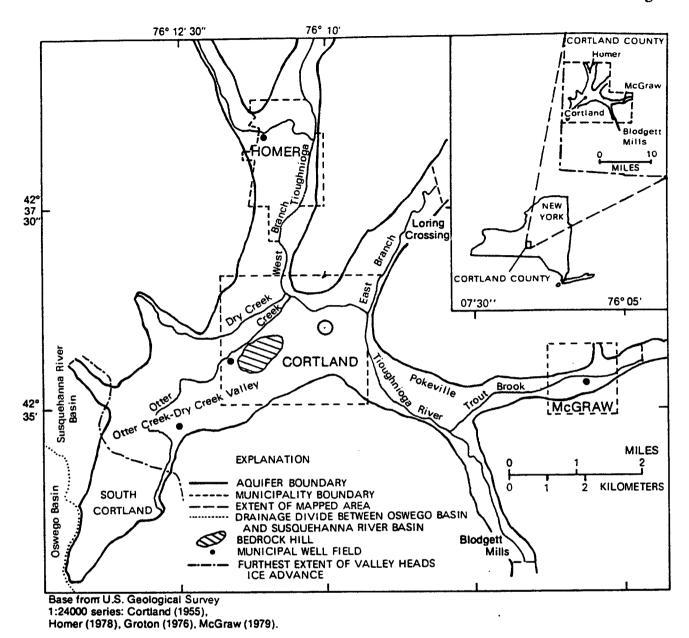


Figure 1.--Location and major geographic features of the study area.

GEOLOGY OF THE CORTLAND AQUIFER SYSTEM

Geologic investigations were conducted from 1979 to 1981 to define the hydrogeologic framework of the valley sections to be added to the Otter Creek-Dry Creek model developed by Cosner and Harsh (1978). This work consisted of a well inventory, a review of drillers' logs, drilling several test holes, and installing several observation wells. (Selected well and test-hole logs are presented in appendix I.) These and other logs were used to construct geologic sections of the area and to aid in developing several of the input data matrices for the model. In addition, a seepage investigation over a 2.84-mile reach of the Tioughnioga River was conducted to evaluate ground-water discharge from the surficial outwash aquifer.

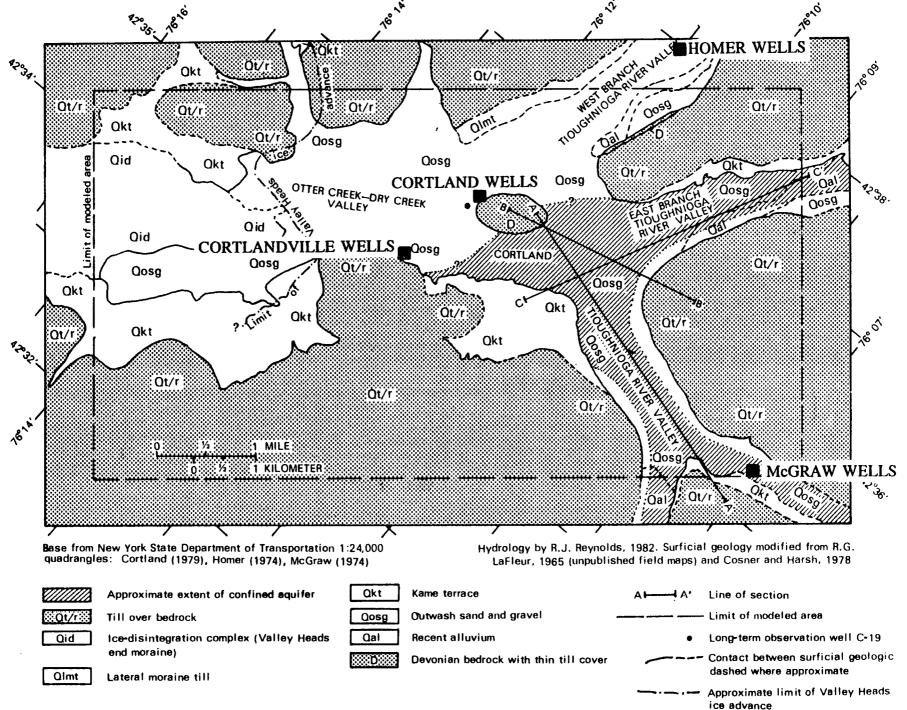


Figure 2. -- Generalized surficial geology and extent of confined aquifer in the Cortland area.

The aquifer of primary interest in the Cortland area is a thick outwash deposit that extends from the Valley Heads end moraine, southwest of Cortland (fig. 2), northeastward throughout the city of Cortland and into the adjacent Tioughnioga River valley (Buller and others, 1978; Cosner and Harsh, 1978). The surficial outwash aquifer is of interest because of its hydraulic connection to small streams that cross it and to major rivers that border it. This outwash deposit is flanked in places by kame terraces composed of ice-contact stratified sand, gravel, silt, and clay; locally it contains discontinuous interbedded lenses of fine-grained silt and clay. The outwash becomes more silty and clayey southwestward and gradually grades into the Valley Heads moraine southwest of Cortland (fig. 2; pl. 1).

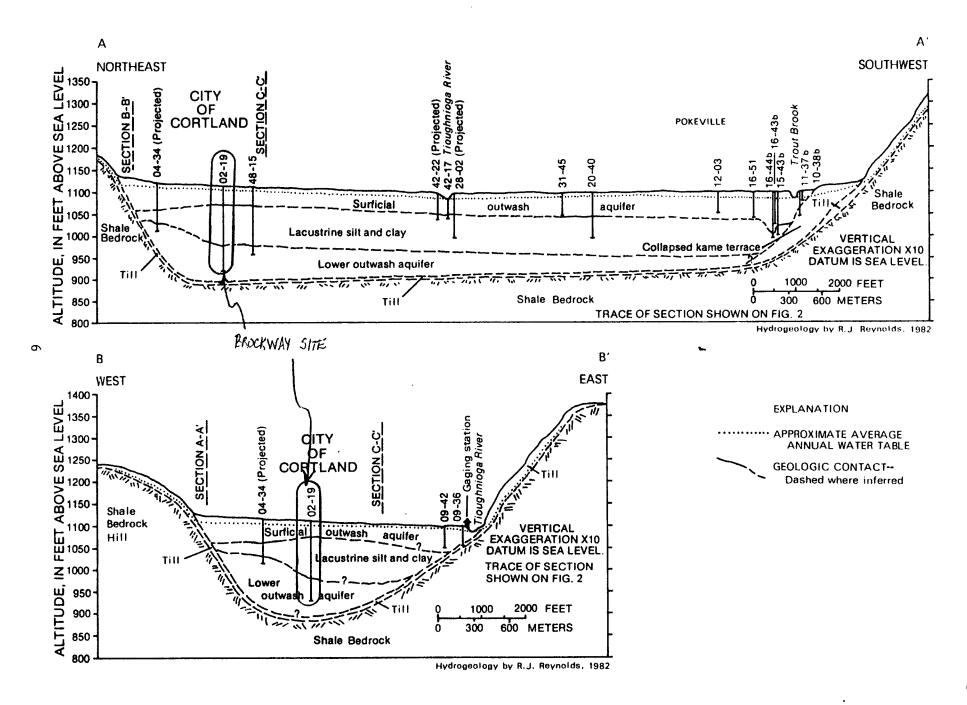
The Valley Heads moraine is an ice-disintegration complex of till and interbedded discontinuous lenses of gravel and sand (Cosner and Harsh, 1978). Surrounding this valley system are till-mantled bedrock hills that rise to a maximum of 700 ft above the valley floor. Within the city of Cortland, a bedrock hill protrudes above the valley floor and partly isolates the Otter Creek-Dry Creek outwash aquifer from the adjacent Tioughnioga River.

Well-completion data (Randall, 1972) and new test-hole data (appendix I, pl. 1) confirm the presence of a thick sequence of lacustrine silts and clays in the Tioughnioga River valley extending from Cortland southeast down the river valley and northeastward up the East Branch (fig. 2). This lacustrine unit, which pinches out a short distance southwest (upvalley) of Cortland in the Otter Creek-Dry Creek basin, separates the surficial outwash aquifer from a confined, deeper sand and gravel aquifer (fig. 2) in these valley limbs. The relative position of these units is shown in cross section in figure 3.

Surficial Outwash Aquifer

The surficial outwash aquifer is hydraulically connected to the overlying East Branch, West Branch, and Tioughnioga Rivers. Because of this connection, pumping from this aquifer at some locations could induce infiltration from these rivers in addition to capturing ground water that would normally enter them; thus possibly reducing flow in the river to unacceptable levels during annual low-flow periods.

In the East Branch and Tioughnioga River valleys, the outwash forms a water-table aquifer consisting of variably silty sand and gravel. In the Tioughnioga River valley between Cortland and Pokeville, the saturated thickness of the aquifer generally ranges from 30 to 50 ft (fig. 3A). Near Trout Brook, southeast of Pokeville, the saturated thickness increases to approximately 80 ft where the lacustrine unit beneath it pinches out and the surficial aquifer merges with the sand and gravel of a collapsed kame terrace (fig. 3A). In the East Branch Tioughnioga River valley (fig. 3C), the surficial aquifer thins from a maximum saturated thickness of 50 ft at Cortland to about 25 ft at Loring Crossing.



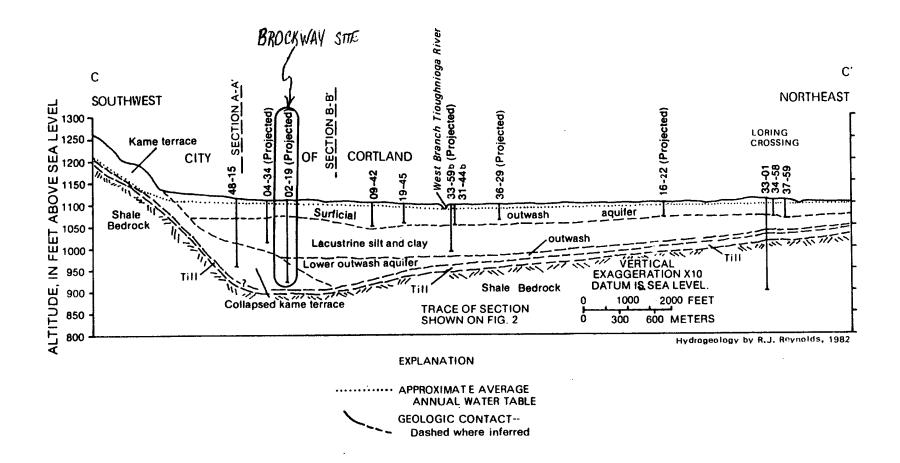


Figure 3.--Geologic sections: A-A', from Cortland to Pokeville along the Tioughnioga River valley near Cortland; B-B', across the Tioughnioga River valley at Cortland; C-C', along the East Branch Tioughnioga River valley from Cortland to Loring Crossing. (Trace of sections is shown on pl. 1 and fig. 2.)

Kame Terraces

Kame terraces are ice-contact, glaciofluvial deposits of stratified sand and gravel with interbedded layers of silt and clay that were deposited by meltwater streams flowing in channels between the glacier and the valley walls. Material was often deposited atop or against the ice bordering these channels, so that when the ice melted, the deposits formed flat-topped, irregularly shaped terraces with components spreading outward into the valley. The two kame terraces that flank the Otter Creek-Dry Creek valley on its southern and northern edges (fig. 2, pl. 1) are hydrogeologically significant because they provide avenues for additional recharge to the outwash aquifer. The buried, collapsed edges of these kame terraces commonly extend downward under the surficial outwash and toward the valley center and may merge with or directly overlie previously deposited outwash. In areas where this collapsed kamic material and outwash is overlain by lacustrine deposits, together they form a confined aquifer that extends into the Tioughnioga River valley (fig. 2). This confined aquifer lies within the eastern part of the Otter Creek-Dry Creek valley and extends from the bedrock hill within the city of Cortland eastward into the Tioughnioga and East Branch Tioughnioga River valleys (fig. 2).

Lacustrine Unit

Lacustrine sediments, which consist of interbedded fine sand, silt, and clay, were deposited in proglacial lakes that developed between older sediment deposits downvalley and the retreating ice front in the Otter Creek-Dry Creek and Tioughnioga River valleys. Meltwater streams carrying a mixture of gravel, silt, and clay under high velocity deposited the coarser fraction of their sediment loads at the edge of these proglacial lakes to form kame deltas, while the lighter fraction, mostly silt and clay, was carried further into the lake, where it gradually settled out to form thick lacustrine units.

Geologic data show that the silt and clay lacustrine unit, which overlies a confined sand and gravel aquifer throughout the East Branch Tioughnioga and Tioughnioga River valleys (fig. 2), is thickest in the Cortland vicinity and thins slightly southeastward toward Pokeville. Section A-A' (fig. 3A) shows the lacustrine unit to be approximately 95 ft thick at the city of Cortland and approximately 80 ft thick near Pokeville. The thickness of the lacustrine unit varies considerably and corresponds to changes in altitude of the underlying kame terrace or outwash surface. The silty clay unit is saturated throughout the study area, but its thickness and low hydraulic conductivity retard upward movement of ground water from the lower aquifer to the upper aquifer. The lacustrine unit thins out southwest of Cortland, as evidenced by the log for well 04-34 (appendix I and fig. 3B), which indicates only 35 ft of this unit.

The underlying aquifer within the city of Cortland probably consists of collapsed and buried material from the large kame terrace south of Cortland (fig. 3B) that extends toward the center of the valley near the southwest city boundary. This collapsed, buried kame terrace provided an elevated surface upon which the lacustrine unit was deposited and thus accounts for the smaller thickness of the lacustrine unit in this area.

The lacustrine unit thins rapidly northeastward from a maximum thickness of 95 ft at Cortland to about 35 ft at Loring Crossing as a result of the upward slope of the underlying units (fig. 3C). Figure 2 shows the approximate areal extent (at depth) of the lacustrine unit and underlying confined aquifer within the valleys.

Confined Outwash Aquifer

Beneath the lacustrine unit in the East Branch Tioughnioga and Tioughnioga River valleys (fig. 2) is a confined sand and gravel aquifer of highly variable thickness that is probably older outwash. Its variable thickness is due, in part, to erosion of its upper surface and its merging with collapsed kame-terrace deposits in some areas; its thickness also reflects the altitude of the underlying till and bedrock surfaces. South of the city of Cortland (fig. 3C), this unit merges with collapsed kame-terrace material to form the thickest confined section, estimated to be approximately 75 ft thick. From Cortland northeast to Loring Crossing (fig. 3C), this buried aquifer thins to less than 10 ft thick as a result of the upward slope of the underlying bedrock surface and probably pinches out farther north in the valley. Geologic data on the thickness of this unit in the Tioughnioga River valley (fig. 3A) southeast of Cortland, although scant, suggest that the unit may thin from 75 ft at Cortland to about 30 ft at Pokeville.

Bedrock and Till

Bedrock in the study area consists of shale units of the Upper Devonian Genesee Group (Rickard and Fisher, 1970). The bedrock configuration (Cosner and Harsh, 1978) is representative of a preglacial drainage network, but its surface configuration is not well known. Depth to bedrock in the area ranges from land surface to more than 200 ft. Where shale is exposed at land surface it is weathered and jointed, but the number and size of openings decrease with depth (Cosner and Harsh, 1978). The joints and bedding planes, which form only a small fraction of the total bedrock volume, are the only significant void space in which water can be stored and transmitted. Thus, shale bedrock in this area is a relatively low-yielding source of water and is used only for farm and domestic supplies.

Till in this area is a poorly sorted mixture of silty clay with varying amounts of sand, gravel, and boulders. A veneer of till directly overlies shale bedrock in the uplands surrounding the study area, where it may range in thickness from 2 to 20 ft. Till is also assumed to overlie bedrock in parts of the valley (figs. 3A. 3B. 3C), but test-hole data are insufficient to confirm this. Till in this region generally has a low hydraulic conductivity and therefore does not transmit or yield water readily. However, sufficient domestic supplies can usually be obtained from shallow, large-diameter dug wells excavated in till. Because of this low permeability, most of the precipitation falling on till in upland areas does not infiltrate but is routed to streams as runoff. However, some recharge does occur in areas of upland till, principally during the winter months, and provides base flow to the small streams that drain these areas (Cosner and Harsh, 1978).

HYDROLOGY OF THE CORTLAND AQUIFER SYSTEM

The valley configuration at Cortland represents a hydrologic situation that is uncommon in New York, in that the aquifers in the Otter Creek-Dry Creek valley are, to a large extent, hydraulically separated from the adjacent Tioughnioga River.

Most of the productive valley aquifers within the Susquehanna River basin and elsewhere in upstate New York are parallel to and have a direct hydraulic connection with streams or rivers that cross or border them, so that heavy pumping from the aquifers, even for relatively short periods, normally reduces adjacent streamflow by induced infiltration. The Otter Creek-Dry Creek outwash aquifer, however, is partly separated from the West Branch Tioughnioga River and Tioughnioga River by a bedrock hill in the center of the valley that serves to disrupt the ground-water flow field between the municipal pumping centers and the river. In addition, the distances from the bordering rivers to the municipal well fields are large enough to ensure that induced infiltration from the rivers would not readily take place. Moreover, the difference between the average altitude of the water table at the Cortland municipal well fields and the average river stage would seem to reduce the likelihood of causing induced infiltration under normal circumstances.

Another type of separated aquifer, a confined outwash aquifer, lies beneath the thick deposits of lacustrine clay and silt within the city of Cortland and in the East Branch Tioughnioga and Tioughnioga River valleys. Confined valley aquifers such as this are hydraulically separated from the overlying rivers by thick lacustrine units, which generally have a very low hydraulic conductivity. Pumping from the confined aquifer in the Cortland area, therefore, would not readily induce infiltration from the Tioughnioga River because the lacustrine unit would inhibit any downward leakage.

System Boundaries

The aquifer system at Cortland consists of variable lithologic units that form a surficial outwash aquifer and a confined outwash aquifer. The study area comprises the Otter Creek-Dry Creek basin and parts of the East Branch, West Branch, and Tioughnioga River valleys.

The aquifer system in the Cortland area is bounded on the southwest by the Valley Heads terminal moraine, which forms the surface-water and ground-water divide between the Oswego drainage basin and the Susquehanna River basin (fig. 1). The Tioughnioga River, on the east and southeast side of Cortland, is the discharge point for much of the ground water moving through the aquifer system. The bedrock valley walls flanking the Otter Creek-Dry Creek basin and the Tioughnioga River are treated as impermeable boundaries because shale bedrock generally transmits little water; in other words, the amount of ground water seeping from the bedrock to the aquifer is negligible compared to the amount contributed by direct precipitation and adjacent stratified drift.

The lacustrine unit in the eastern part of the Otter Creek-Dry Creek basin and in the Tioughnioga River valley (figs. 3A, 3B, 3C) overlying the confined outwash aquifer (fig. 2) acts as a confining unit and produces artesian conditions in the underlying aquifer.

Recharge

The aquifer system at Cortland is recharged primarily through infiltration of precipitation, although some recharge occurs as leakage from streams, as ground-water flow from flanking kame-terrace deposits, and from runoff from adjacent bedrock hills. Rates of recharge to stratified-drift aquifers such as these vary seasonally and areally and must be estimated because they cannot be measured directly. Randall and others (1966) show that average annual recharge to stratified drift may reach 1 (Mgal/d)/mi². Cosner and Harsh (1978) used a recharge rate of 28 in/yr, which equals approximately 1.25 (Mgal/d)/mi². Over a modeled area of 7.9 mi², this amounts to 9.89 Mgal/d of recharge. Although about 30 percent of the average annual precipitation (41 in.) falls during the growing season (Sealy and others, 1961), nearly all of it is lost through evapotranspiration, and only a small amount recharges the aquifer (MacNish and others, 1969).

Surficial aquifer

The surficial aquifer is recharged primarily by precipitation and by streambed leakage over losing reaches of small streams in the Otter Creek-Dry Creek basin and probably elsewhere in the valleys. Recharge from Otter Creek and Dry Creek occurs along the stream channels where they traverse the valley floor, from the point where the streams leave the till-covered bedrock uplands to their confluence with the West Branch Tioughnioga River (Cosner and Harsh, 1978; Buller and others, 1978).

Additional recharge to the surficial aquifer also occurs through the kame terraces that flank the north and south edges of the Otter Creek-Dry Creek basin (fig. 2). These kame terraces are hydraulically connected to both the surficial and confined aquifers, and precipitation on these kame terraces therefore recharges both the upper and lower outwash aquifers.

Confined aquifer

The confined outwash aquifer, which underlies the eastern end of the Otter Creek-Dry Creek basin and extends throughout the Tioughnioga River valley, is recharged wherever it is hydraulically connected with the upper aquifer—that is, wherever the intervening lacustrine unit is absent. The recharge area for the confined aquifer is assumed to be west of the bedrock hill in the western part of the Otter Creek-Dry Creek basin (fig. 2) because the lacustrine unit in this area pinches out, thus allowing good hydraulic connection between the two aquifers through which recharge can occur. Additional recharge to the confined aquifer occurs where the aquifer is hydraulically connected to kame terraces at the valley sides, such as near the south side of Cortland and near Pokeville (figs. 2, 3A, 3C).

Ground-Water Movement

Information about directions of ground-water flow and seasonal watertable configurations was obtained from periodic water-level measurements in a network of observation wells within the Otter Creek-Dry Creek basin during 1976 (Cosner and Harsh, 1978). Additional information about general ground-water flow directions and water-table fluctuations in the extended-model area was obtained during 1980-81 from periodic water-level measurements in two observation wells and from historical water-level data. The 1976 water-level data and the 1980-81 data together provided the basis for a steady-state calibration of the enlarged model.

Ground-water movement in the surficial outwash aquifer within the Otter Creek-Dry Creek basin is generally northeastward (downvalley) toward the West Branch Tioughnioga and Tioughnioga Rivers and moves eastward, more perpendicular to the river, within the city of Cortland. Ground-water flow in the East Branch Tioughnioga River valley is predominantly eastward across the valley toward the East Branch Tioughnioga River, and flow in the Tioughnioga River valley between Cortland and Pokeville is westward across the valley toward the Tioughnoiga River. Along some valley walls that border the aquifer, such as southwest of Cortland, ground water from kame terraces flows into both the surficial aquifer and confined aquifer. General directions of ground-water flow in the surficial aquifer during spring conditions are shown on plate 2.

Historic water-level data on the lower outwash aquifer are scant; however, hydraulic heads in this aquifer are probably above the water table in most areas within the Tioughnioga valley and seasonally above land surface in the vicinity of Cortland. Records for well 48-15, drilled in 1944 for Brewer-Tichner Corp. (pl. 1, figs. 3A, 3C) and screened in the lower unit, indicate that the well flowed for approximately 1 month after completion (Randall, 1972). Because the principal recharge area for the confined aquifer is in the Otter Creek-Dry Creek basin, ground-water flow in this aquifer is probably northeastward from the recharge area toward the Tioughnioga River, then southeastward in the Tioughnioga River valley. The discharge area for ground water moving through the confined aquifer is assumed to be further south in the Tioughnioga River valley, beyond the study area.

STREAM-AQUIFER RELATIONSHIP

Several factors determine the rate of ground-water flow between the surficial aquifer and the overlying rivers or streams. The most important of these are (1) the head difference between the stream (or river) stage and the underlying aquifer, (2) the vertical hydraulic conductivity of the streambed material, (3) the hydraulic conductivity of the aquifer, (4) the depth of incision of the stream into the aquifer, and (5) the proximity of nearby impermeable boundaries, such as a bedrock wall, which may alter ground-water flow paths near the river.

In this study, an effort was made to evaluate the amount of ground-water seepage to the Tioughnioga River and to estimate the hydraulic conductivity of the streambed and the aquifer. This was done by measuring discharge at successive points along the Tioughnioga River near Cortland to determine areas of gains or losses of flow (generally known as a seepage investigation), combined with water-level measurements at nearby wells and augmented by analysis of specific-capacity data from a nearby industrial well screened in the surficial

station during the seepage run. For expediency, it is assumed that this total head loss is measured across a streambed thickness of 1.5 ft, as used by Cosner and Harsh (1978). The vertical hydraulic conductivity of the streambed, based on measured seepage over stream reach B, was calculated to be 1.04 ft/d, which is comparable to the value of 1.9 ft/d reported by Haeni (1978) for the Pootatuck River in Connecticut but is two orders of magnitude greater than the value of 0.038 ft/d used by Cosner and Harsh (1978). The value selected by Cosner and Harsh was based solely on model calibration to achieve a desired total stream-leakage value for Otter and Dry Creeks. As such it is dependent on the accuracy of the estimates of other hydrologic variables used as model input.

Aquifer Characteristics

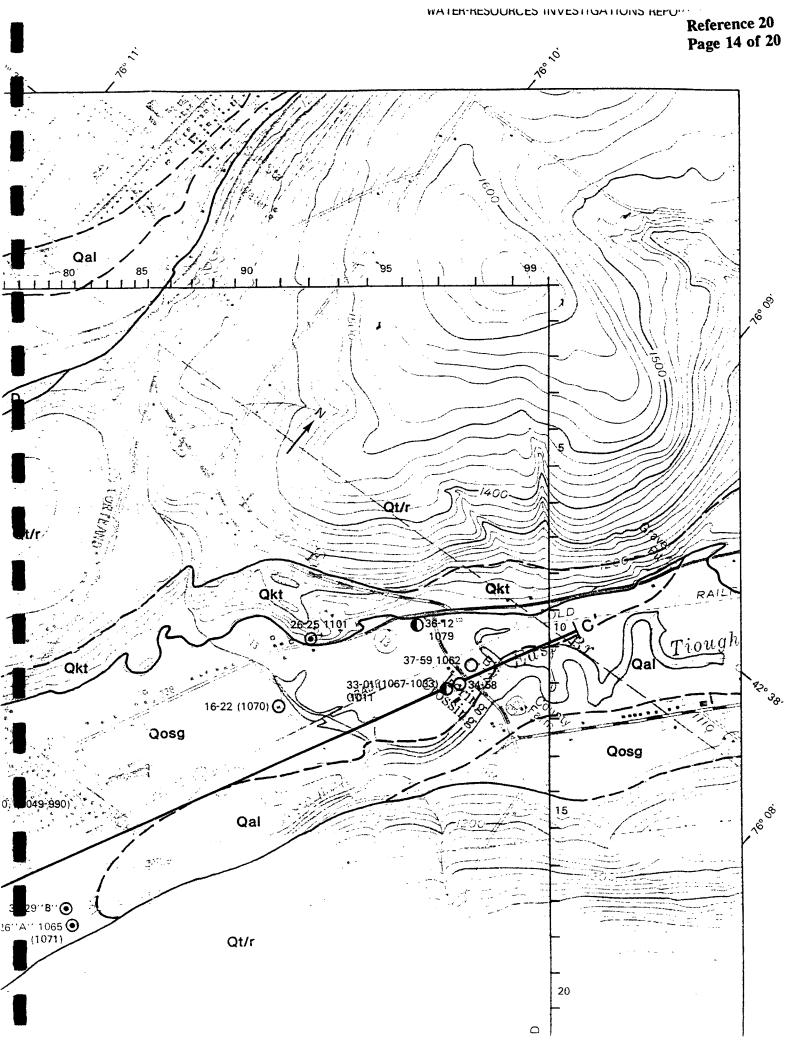
The surficial aquifer is highly permeable, with a hydraulic conductivity of 300 to 350 ft/d as estimated from specific-capacity data from an industrial well near Pokeville in the Tioughnioga River valley (appendix II). These values and the range in saturated thickness indicate that the transmissivity of the surficial aquifer in the Tioughnioga River valley ranges from approximately 7,500 to 28,000 ft/d. Most 6-inch-diameter domestic wells tapping this aquifer are finished open ended (with no well screen); therefore, reported yields are commonly less than 50 gal/min. However, properly screened and developed production wells tapping this unit could be expected to yield at least 500 gal/min (provided the saturated thickness is adequate), as evidenced by the high specific capacity of 44 (gal/min)/ft at a pumping rate of 350 gal/min at the 12-inch-diameter industrial well near Pokeville mentioned above.

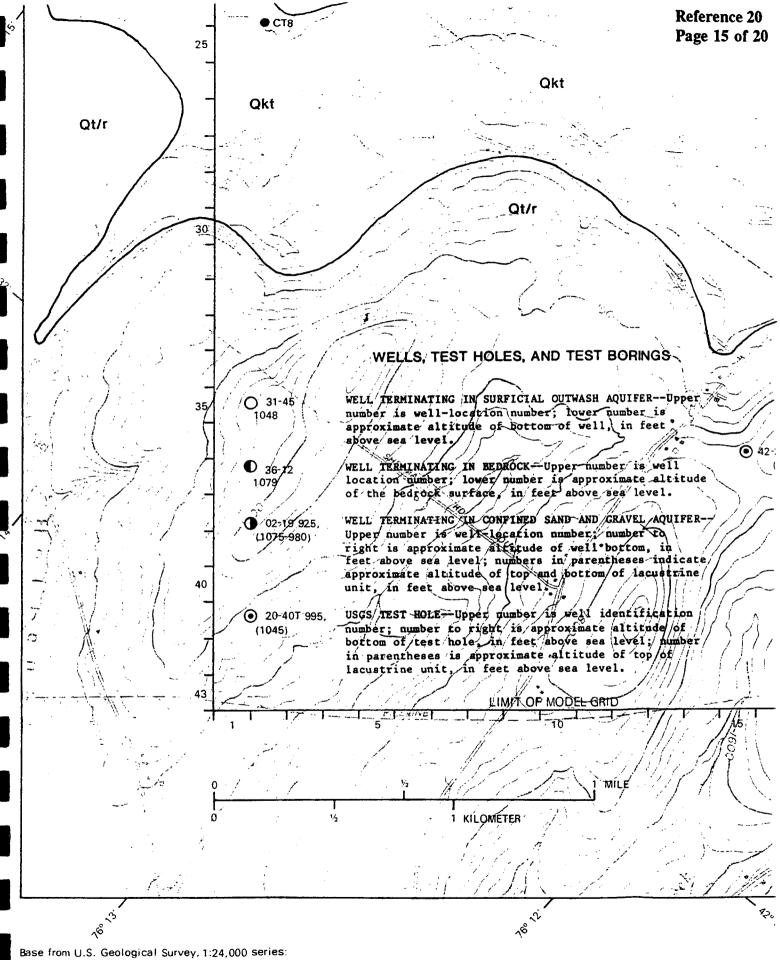
The hydraulic-conductivity value obtained for the surficial aquifer in this area is substantially lower than those calculated from three wells tapping the same aquifer in the Otter Creek-Dry Creek valley. On the basis of three aquifer tests, Buller and others (1978) reported transmissivity in the Otter Creek-Dry Creek basin to range from 37,000 to 80,000 ft2/d, with respective hydraulic-conductivity values ranging from 950 to 1,140 ft/d. These data indicate the surficial outwash aquifer to be somewhat less permeable in the Tioughnioga River valley than in the Otter Creek-Dry Creek basin.

SIMULATION OF GROUND-WATER FLOW

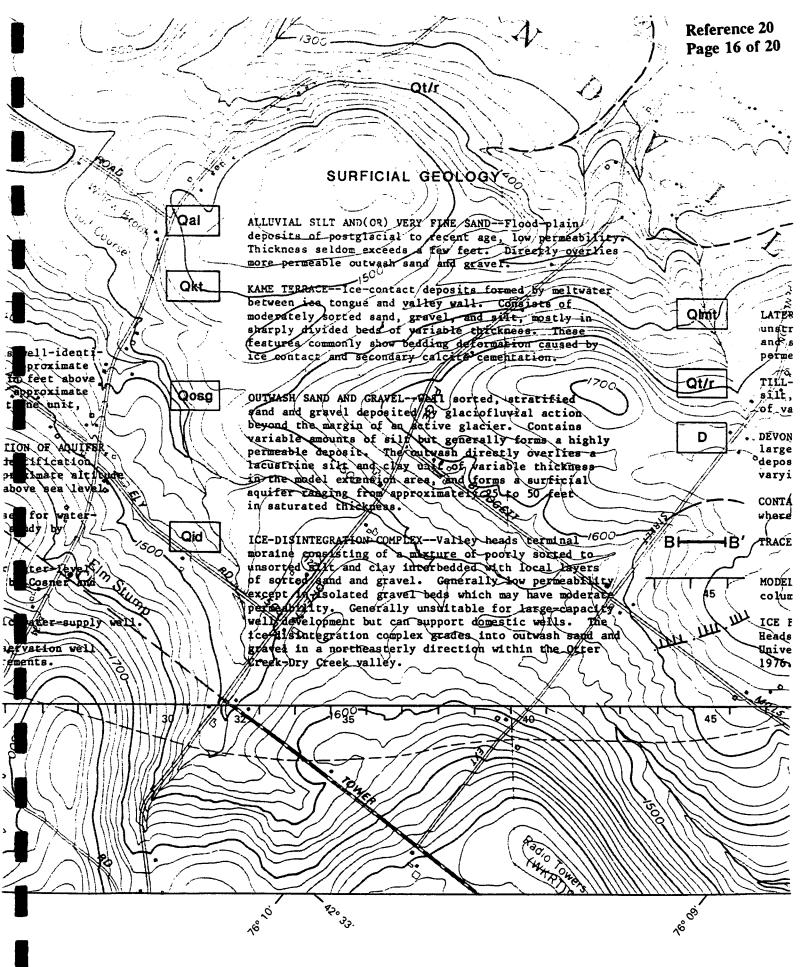
Model Description

A finite-difference ground-water flow model developed by Trescott and others (1976) was used to simulate the response of the surficial outwash aquifer to imposed stresses. The model simulates two-dimensional ground-water flow in response to artifically imposed stresses such as pumping or to naturally occurring stresses such as drought. Given specific values for aquifer characteristics such as hydraulic conductivity and specific yield, the model can be used to simulate water levels that would result under both steady-state (no change in heads with time) and transient-state hydrologic conditions and to calculate changes in water levels that would result from pumping at specific sites. Sources of water may include aquifer storage, recharge from precipitation, inflow across aquifer boundaries, and induced

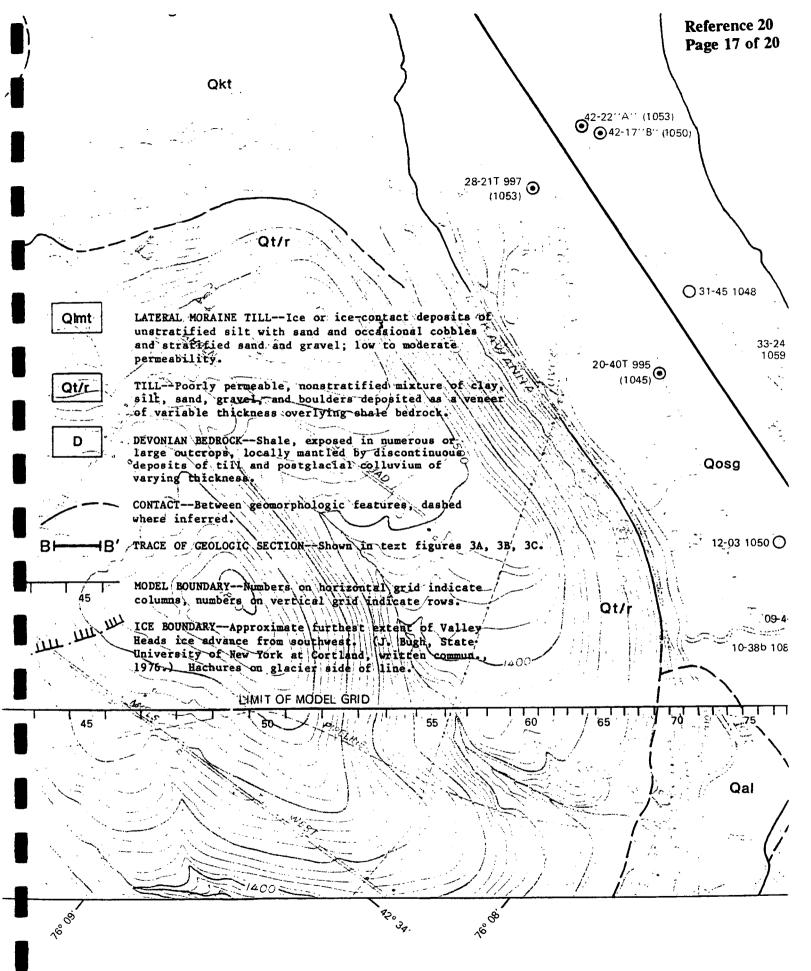




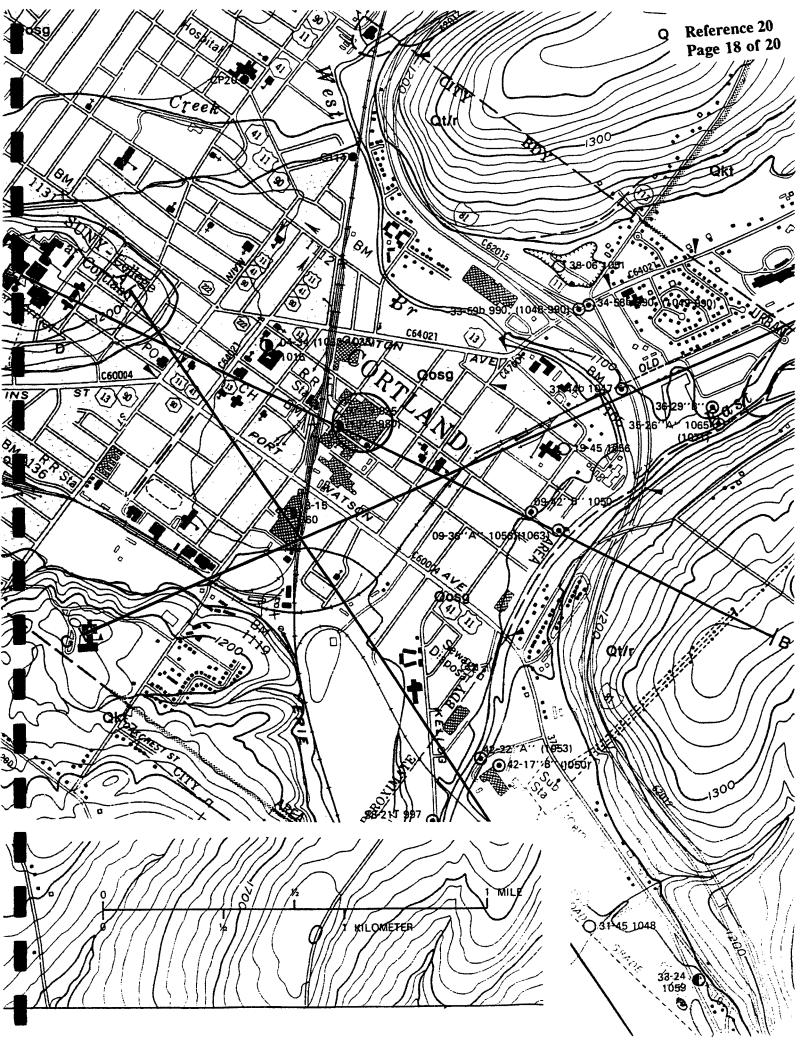
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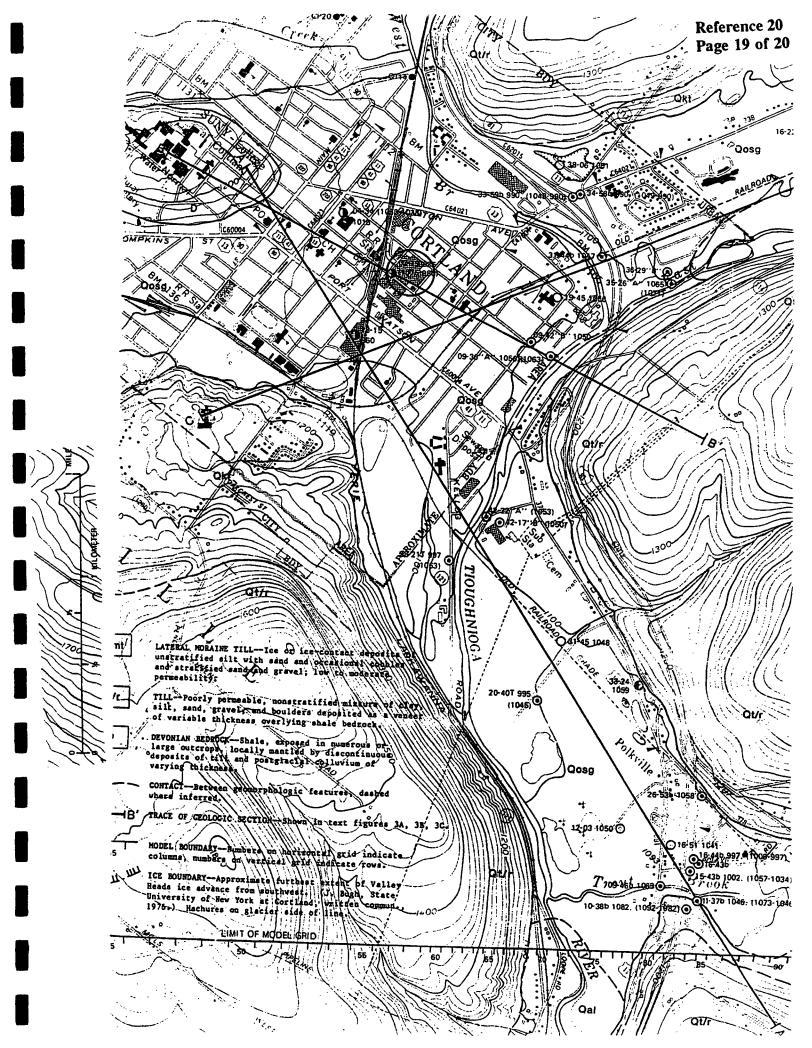


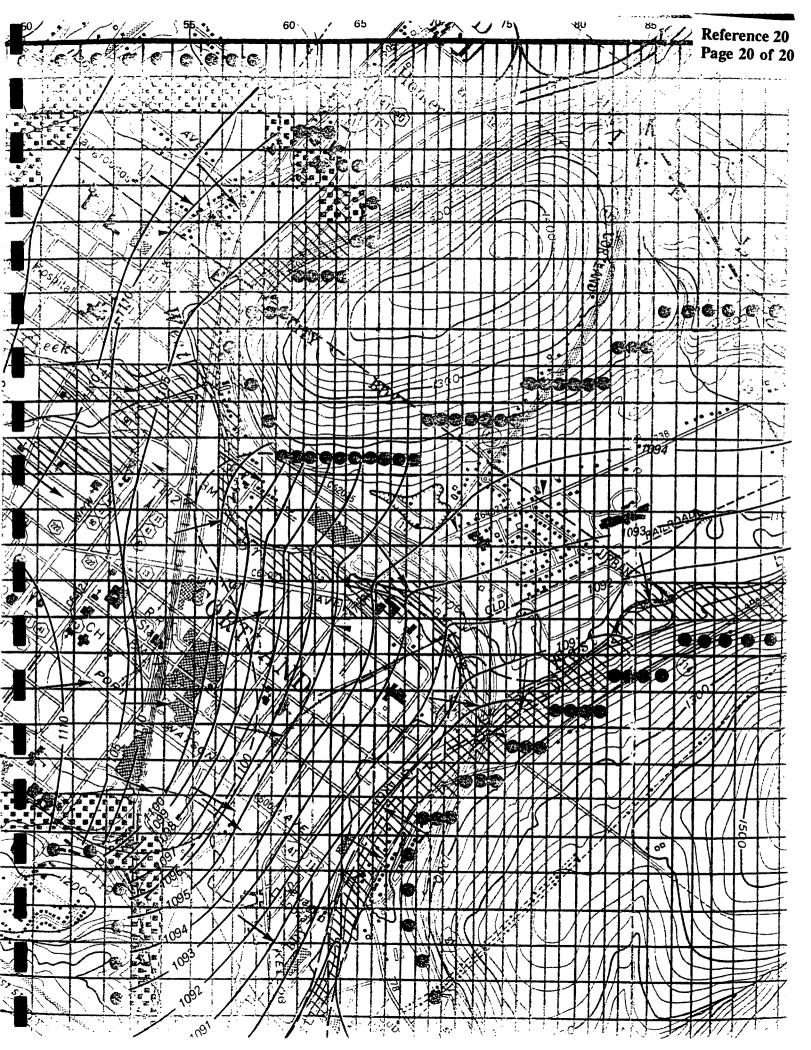
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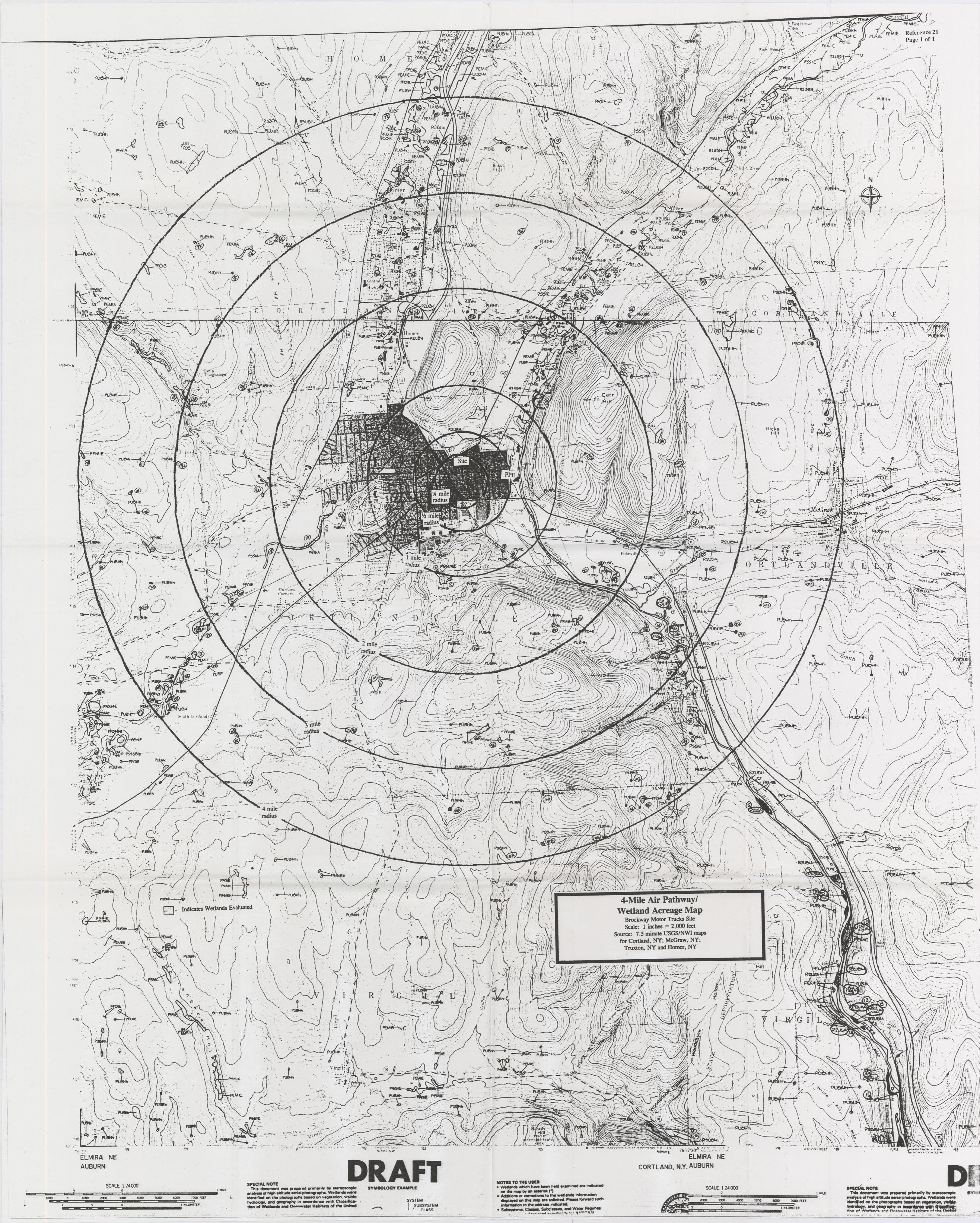


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January 24, 1995

Ebasco Environmental 2111 Wilson Boulevard, Suite 435 Arlington, VA 22201-3058

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Attn: Jeff Martin

We have the following information you requested, according to our deed file records, in the Real Property Tax Services Office.

86.52-03-04 The Cortland Corporation, formerly Brockway Motor Company, Inc. to Mack Trucks, Inc. 1-4-60 Mack Trucks, Inc. to Canford Manufacturing Corporation 2-26-80 Canford Manufacturing Corporation to Cortland County Industrial Development Agency 4-1-80

86.51-02-31 Durkee's Bakery, Inc. to Mack Trucks, Inc. 10-26-73
Mack Trucks, Inc. to SCM Corporation 11-27-78
SCM Corporation to Lawrence P. Brooks, Robert Lacey, Frederick G.
Compagni 4-4-86
Lawrence P. Brooks, Robert Lacey, Frederick G. Compagni to
Richard C. DiCicco and Carmine Todisco 12-24-86
Richard C. DiCicco and Carmine Todisco to Richard C. DiCicco and
Carmine Todisco 2-27-87

86.51-03-07.2 Mack Trucks, Inc. to Michael Brown, d/b/a Round House Mill 9-8-78
Michael Brown to New York State Electric and Gas Corporation 6-3-81

86.52-03-02 Mack Trucks, Inc to Paul A. Sepe and Georgianna T. Sepe 3-7-79
Paul A. Sepe and Georgianna T. Sepe to Canford Manufacturing Corporation 6-4-85

86.60-02-01.1 Mack Trucks, Inc. to Frederick G. Compagni 2-26-80
Frederick G. Compagni to Canford Manufacturing Corporation 3-20-85
Canford Manufacturing Corporation to Rubbermaid-Cortland
We do not have a deed for this transfer.
Rubbermaid-Cortland to Cortland County Industrial Development Agency 8-25-88

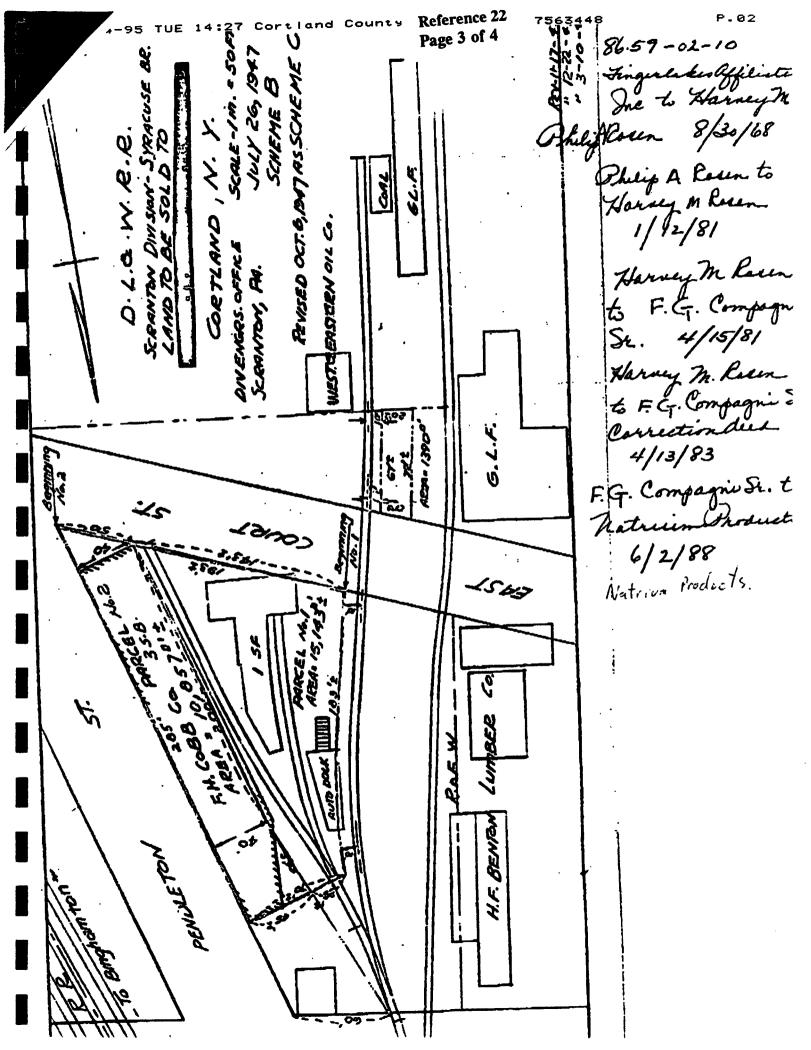
86.51-03-07.1 Mack Trucks, Inc. to Michael Brown, d/b/a Round House Mill 9-8-78

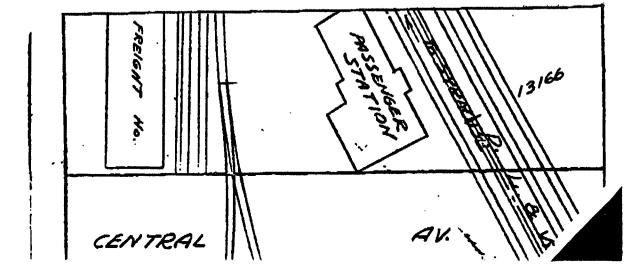
86.59-02-10 Our records do not go back far enough to show this was a portion of Mack Trucks, Inc.

86.52-03-08.2 This is not a valid number.

86.60-02-03 (now consolidated with 86.60-02-01.1) Mack Trucks, Inc. to Joseph Compagni and Jane M. Compagni and Joseph H. Compagni &-27-79

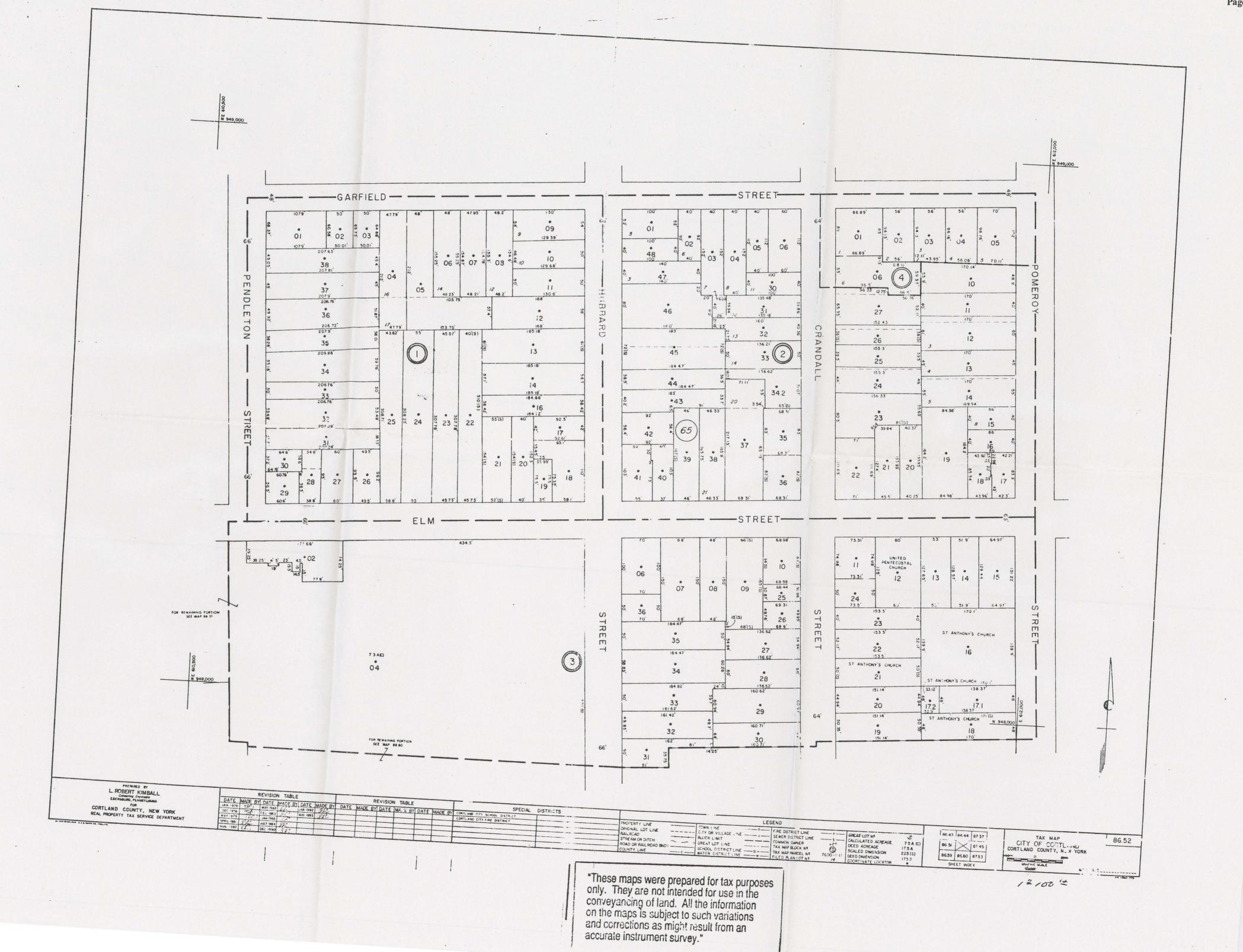
Joseph Compagni and Jane M. Compagni and Joseph H. Compagni to Canford Manufacturing Corp. 8-12-85

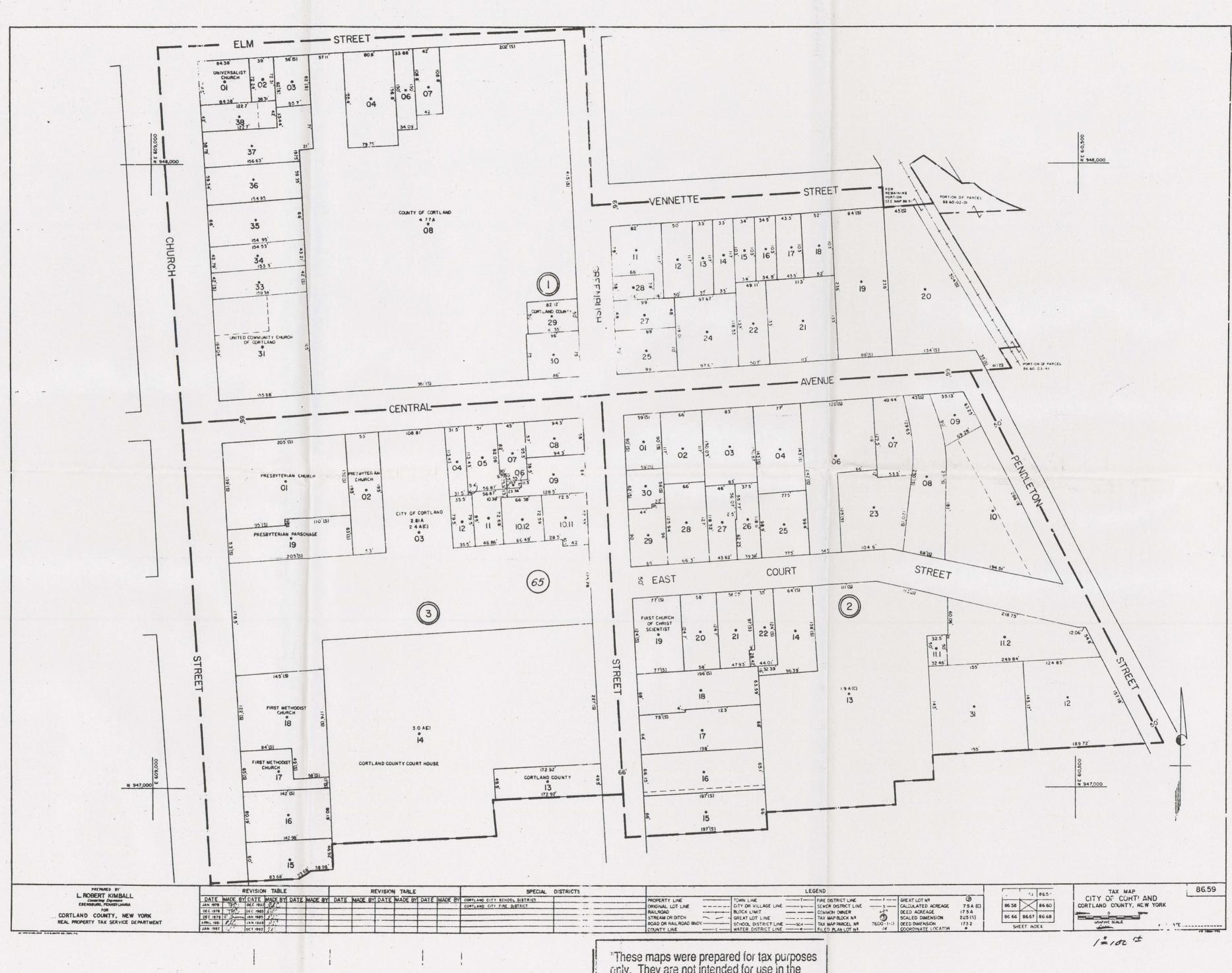




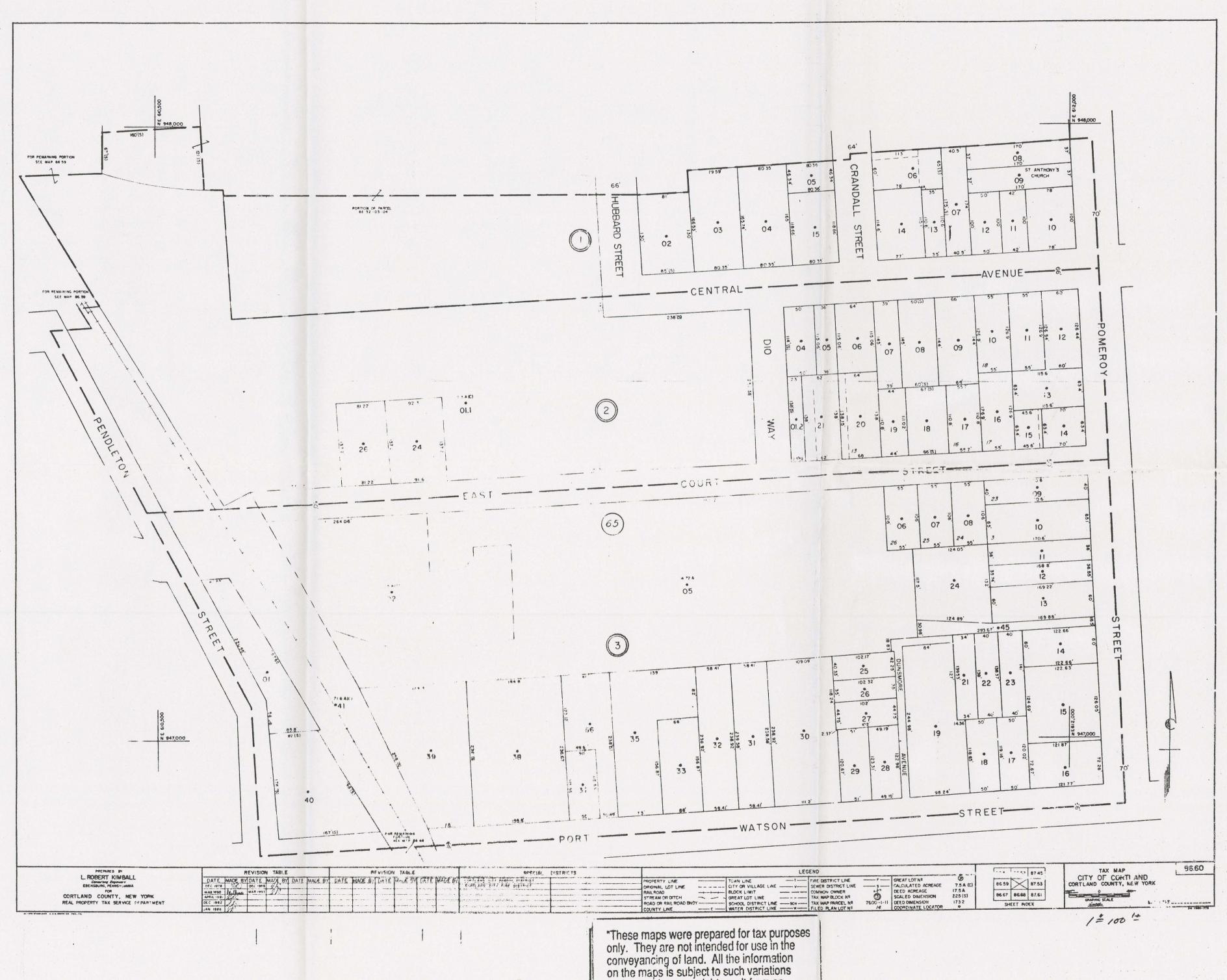


These maps were prepared for tax purposes only. They are not intended for use in the conveyancing of land. All the information on the maps is subject to such variations and corrections as might result from an accurate instrument survey."





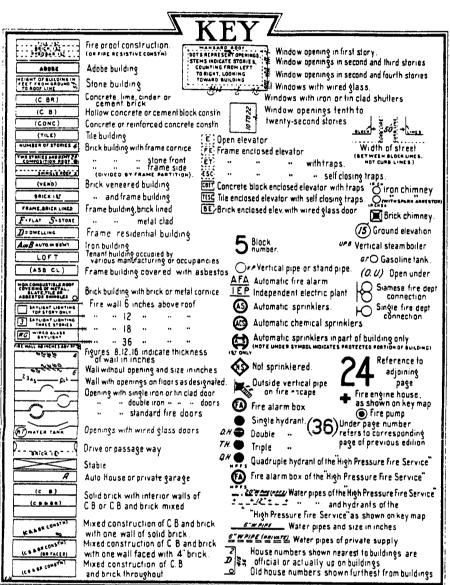
"These maps were prepared for tax purposes only. They are not intended for use in the conveyancing of land. All the information on the maps is subject to such variations and corrections as might result from an accurate instrument survey."



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C. Protected Steel Frame. D. Individually Protected Steel Joists, Columns, Heams, Trusses, Arches. E. Indirectly Protected Steel Frame. F. Indirectly Protected Steel Joists, Columns, Beams, Trusses, Arches. G. Unprotected Steel Frame. H. Unprotected Steel Joists, Columns, Heams, Trusses, Columns, Trusses, Columns, Heams, Trusses, Columns, Heams, Trusses,	b. Concrete on Metal Lath, neombustible Form toards, Paper-backed fite Fabric, Steel Deck, or Cellular, Ribbed or Corrugated Steel Units. Spen Steel Deck or Grating. C. Incombustible Communication, Massurry or Metal d. Steel Deck, Corrugated Steel d. Steel Deck Corrugated Stee	Planks. Imm on industible per-backed in Deck, bear of thouse and reinfo floors and roof. In the control of thouse and control of t	Since concrete frame, Since Fire ins. Companies. Since F
MASONRY CONSTRUCTION MASONRY CONSTRUCTION SEly, Store pipe.			
(CARA) Cur Fac 2005 18" A 20" Stone	Concrete 1st Flour on	Wall with Double Standard Fire Doors 1st Floor Wall with Standard Fire Door Basement Wall with Substandard Fire	(Exterior) Int Floor Lat Floor Lat Ploor Later Ploor
(CAR) United Respective Respective Respective Priors [C BR] Cinder, Concrete or Concrete	obe CLUC Hollow Cinder or Concrete Block 1st Floor only How Cinder or Concrete Block Interior Block Interior Block Interior Block Busherior Block Busherior Block Busherior Block Busherior Wall Sement to Roof FUCUL Tile 1st & 3rd Floors	Wall with Metal & Wired Glass Fire Doors all Floors Wall with Substandard Fire Doors 1st, 2nd & 3rd Floors & Unprotected Opening 4th Floor Wall with Small Unprotected Openings only	Metal Statter 1st, masonry, conterete, and/or protected steel. F. I. X. F. P. qualifications except inferior or sub-standard walls. N. C. Fire resistive with unprotected structural sivel units. MICH. CON WALL, A banded masonry wall having a continuous air space within, [F. I. Interpretated Electric Plant.] MPASSAILE Nottraversable due to condition of terrain.
Wall with Unprotected Cpenings all Flores NON - MASONRY CONSTRUCTION Non-masonry walls are shown with fine () lines. (Wall construction other than would and stucce on wood frame is noted) Wood & Stucce & Cement Wood			
(VEND) Frame (Other Types of Veneered on Wood Frame Specifically Noted) (IR CL) Met Fra Masonry & Non-Masonry (Type of Masonry Specifically Noted)	rial Sash & Glass rial Clad on Wood ainte	od Stucco, Cement Plaster, Etc., on Steel Frame (A.R.M.) As wood of Frame	Si., Cl., Slate attached to wood siding. SM. HO. Smuke House. STABLE Shown by crossing diagonal lines on diagram. SUSPD. Suspended ceilings below floor and/or rouf beams. SYST. System. TRANSE. Transformer. WID. Wood.
Yachi ¹³ Wood, Brick Lined, Br.	wetal Panels	(GIASS) Gla	ASS PANELS C COMMERCIAL U ULLET " FF onestimate T Years Action Michael Property for the series of Paragraphy of Commercial Commer
FIRE PRO Fire Department Connection Automatic Sprinklers throughout contiguous sections of single risk Automatic Sprinklers all floors of building Automatic Sprinklers in part of mulding only (Note under Symbol indicates profected portion of building) Not Sprinklered Automatic Chemical Sprinklers Chemical Sprinklers in part of building only (Note under Symbol indicates profected portion of building only (Note under Symbol indicates profected portion of building) VP NYD Vertical Pipe or Stand Pice AFA Automatic Fire Alarm	Single Hydrant OH Double Hydrant TH Triple Hydrant QH Quadruple Hydrant of the High Pressure Service Water Pipes of the High Pressure Service Water Pipes of the High Pressure Service as Shown on Key Map Public Water Service VERTICAL OPENINGS Skylight lighting top story only Skylight lighting 3 stories	Frame Enclosed Elevator with Self Closing Traps Concrete Block Enclosed Elevator with Traps Tile Enclosed Elevator with Self Closing Traps Bef Hrick Enclosed Elevator with Wired Glass Door Open Hoist Hoist with Traps Open Hoist Hasement to 1st Stairs MISCELLANEOUS Number of Stories Height in Feet Composition Roof Covering	2 Stories & Basement 1st Floor Occupied by Store 2 Residential Units above 1st Auto in Basement 1prive or Passageway Wood Shingle Hoof Iron Chimney Iron Chimney Iron Chimney Iron Chimney Iron Chimney With Spark Arrestor) Wertical Steam Boiler Wortical Steam Boiler Width of Street between Block Lines, not Curb Lines Ground Elevation House numbers nearest to Buildings are Official or Actually up on Buildings. Old House Numbers are Farthest from Huildings
Water Tank Outside Vertical Pipe on fire cooure Fire Alarm Hox Noted "HPFS" on High Pressure Fire Service	Skylight with Wired Glass in Motal Sash (Yen Elevator Frame Enclosed Elevator with Traps	Frame Cornice Parapet 12" above Roof W HO. WHO. Shingle Roof Covering	Reference to Adjoining Page Number Pire Department as shown on Key Map Vac. or V Vacant Vac. 4 Op. or VO, - Vacant & Open



G.F. Gasoline Tank TANKS 4' EARTH DIKE CRLIDE QIL TANKS CAPCX 100,000 GALS EACH 23 25 2000 GAL 1000 GAL KEROSENE TK. GASOLINE TK $\bigcirc G.T$ E. 20000 GAL PRESSURE TANK ELEVD 20 ABV ROOF ON STEEL FR. 10,000 GAL STEEL W FUEL OIL LINE Fire Cistern CISTERN 2 GARAGE CAPCY. 20 CARS CONG. FL. WOOD RAMPTO 2ND REP. 2ND. PRIVATE GARAGE CAPCY. 10 CARS

CONC. FL.

CODING OF STRUCTURAL UNITS FOR FIREPROOF AND NON-COMBUSTIBLE BUILDINGS FRAMING FLOORS ROOF CODE STRUCTURAL UNIT CODE STRUCTURAL UNIT CODE STRUCTURAL UNIT Reinforced Concrete Reinforced Concrete. Reinforced Concrete. Frame. Reinforced Concrete with Reinforced Concrete with Masonry Units. Masonry Units. Reinforced Concrete Pre-cast Concrete or Reinforced Gypsum Concrete. Joists, Columns, Beams, Gypsum Slabs or Planks. Trusses, Arches, Masonry Pre-cast Concrete or Gypsum Slabs or Planks. Concrete on Metal Lath, Protected Steel Frame. Incombustible Form b. Concrete or Gypsum on Metal Lath, Incombustible Form Boards, Paper-backed Boards, Paper-backed D. Individually Protected Wire Fabric, Steel Deck, Steel Joists, Columns, and Cellular, Ribbed or Wire Fabric, Steel Deck, Beams, Trusses, Arches. Corrugated Steel Units. and Cellular, Ribbed or Indirectly Protected Steel Corrugated Steel Units. Frame. Open Steel Deck or Grating. Incombustible Composition Indirectly Protected Steel LAND USE CODE APPLICABLE TO CHANGES DIAGRAMMED ALTER 5/69 Boards with or without Joists, Columns, Beams, MANUFACTURING Insulation. R RESIDENCIAL Trusses, Arches. Masonry or Metal Tiles. G. Unprotected Steel Frame. RESIDENTIAL -RT PUBLIC OR INSTITUTIONAL P Steel Deck, Corrugated Unprotected Steel Joists. COMMERCIAL UTHITTY Columns, Beams, Trusses, Metal or Asbestos Protected Metal with or Arches. W WAREHOUSE TRANSPORTATION without Insulation. Ο. Masonry Bearing Walls. NUMERICAL PREFIX INDICATES THE NUMBER OF ESTABLISHMENTS IN EACH CATEGORY

The coding for framing, floor and roof structural units as shown above is used in describing the construction of fire-resistive buildings. In addition, reports for fire-resistive buildings will show the date built and wall construction when other than brick.

F P buildings have masonry floors and roof; concrete and/or directly or indirectly protected steel framing; and clay brick, stone or poured concrete walls.

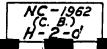
FPX buildings are FP buildings with inferior walls such as concrete block, cement brick, metal or glass panels, etc.

N C buildings have unprotected steel framing and fire-

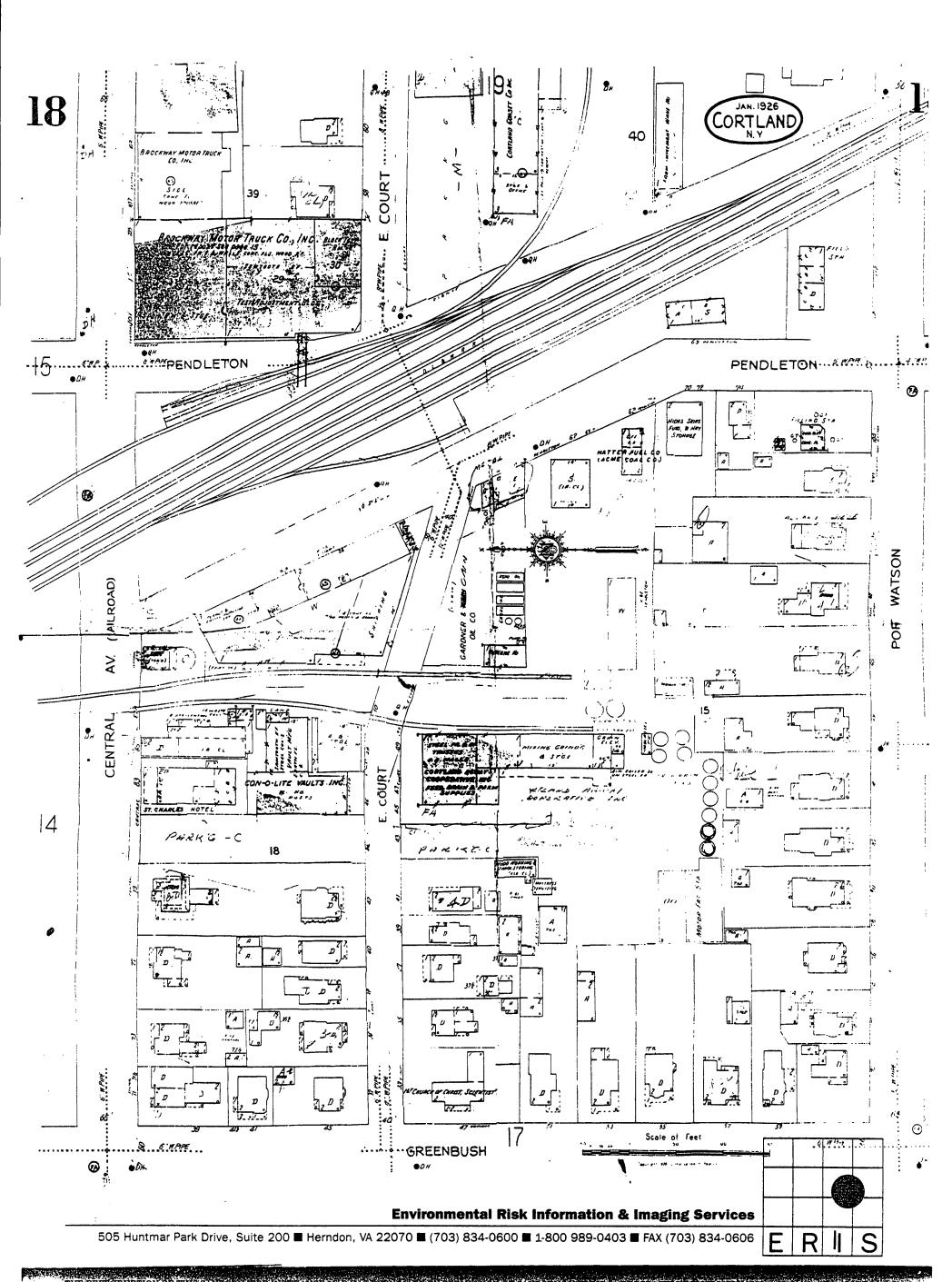
FP-1962 (conc.) A-1-a A fire-resistive building built in 1962 with concrete walls and reinforced concrete frame, floors and roof.

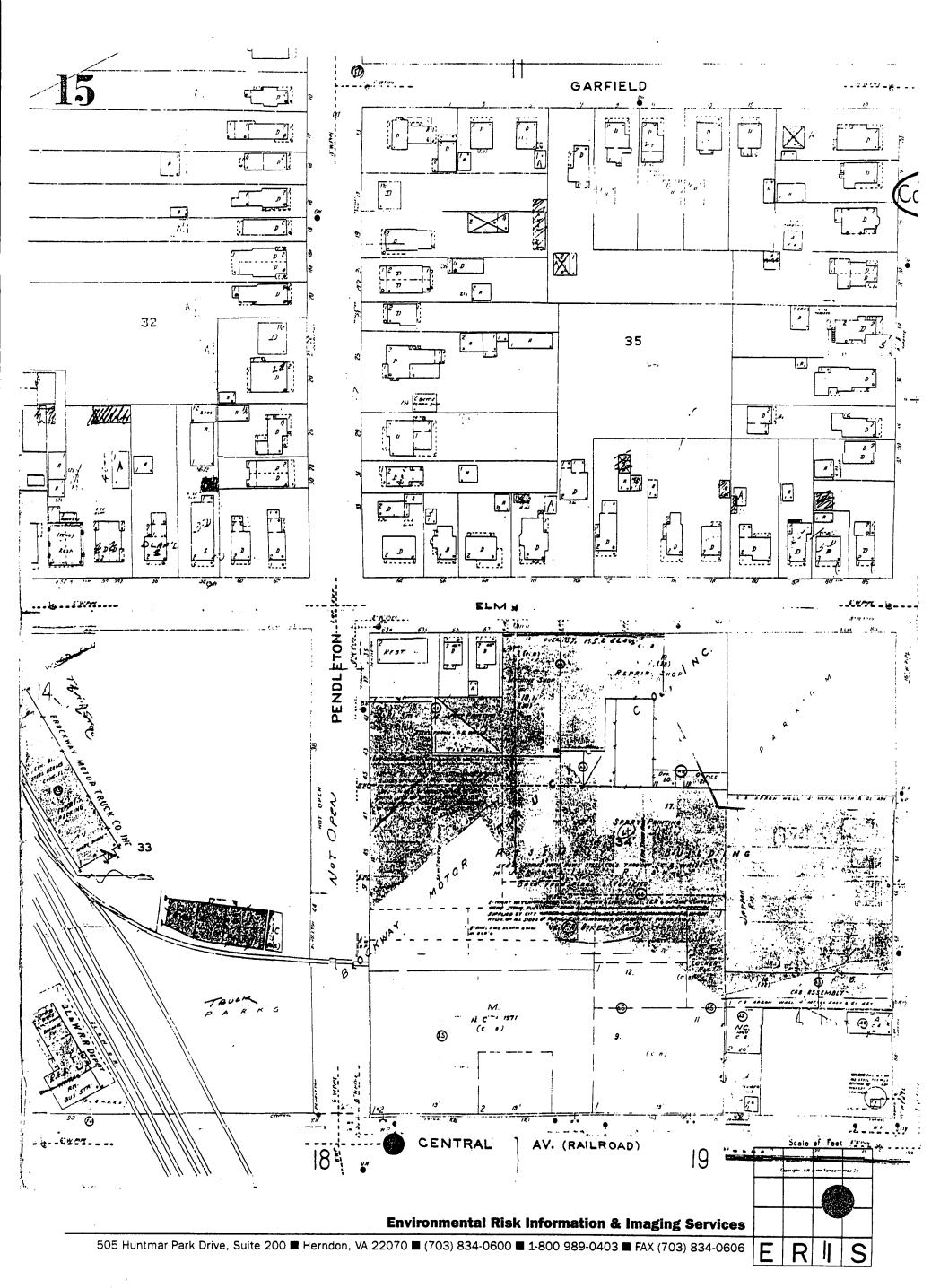


A fire-resistive building built in 1962 with metal panel walls, indirectly protected steel frame, concrete floors and roof on metal lath, noncombustible ceilings.

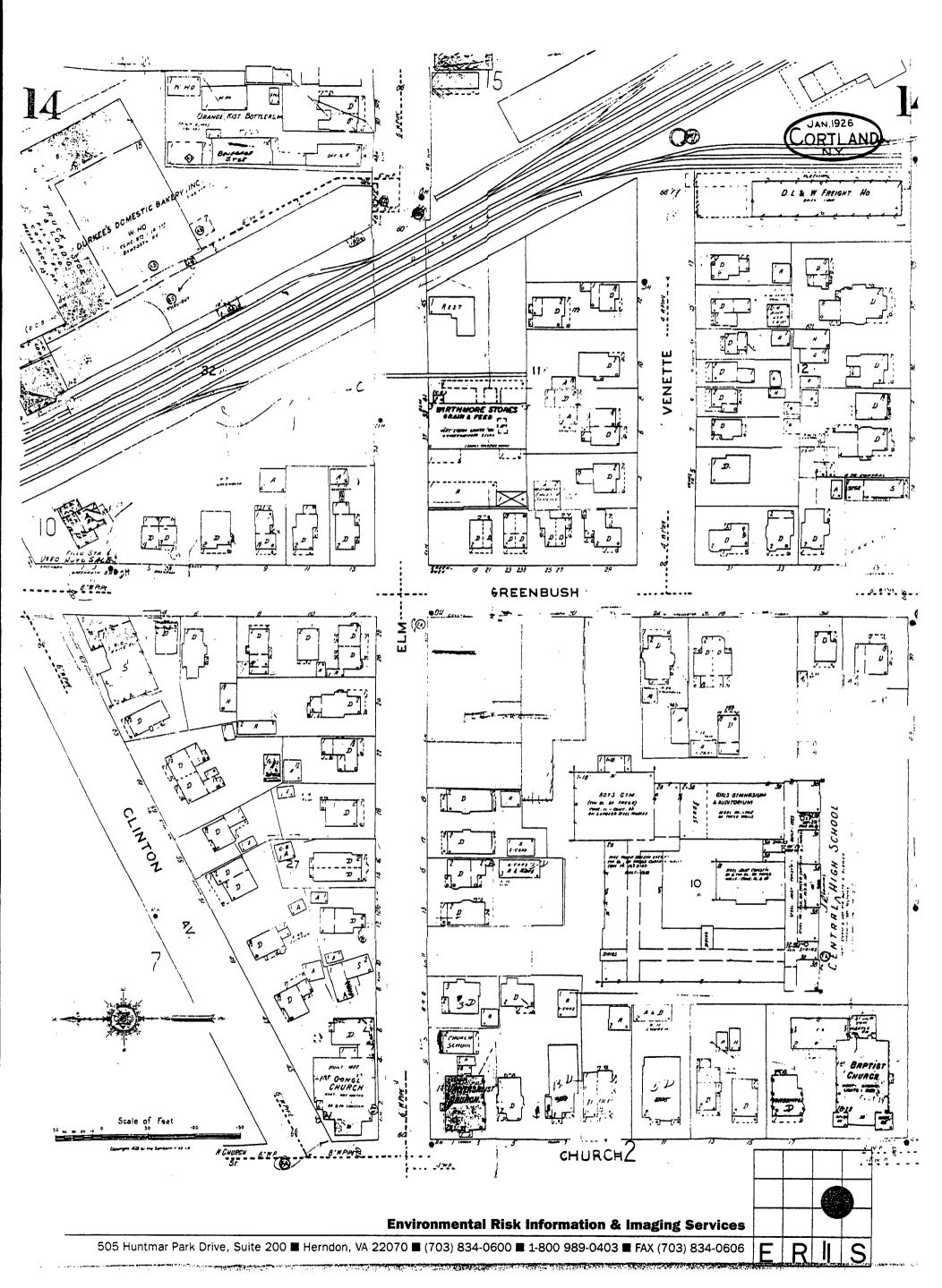


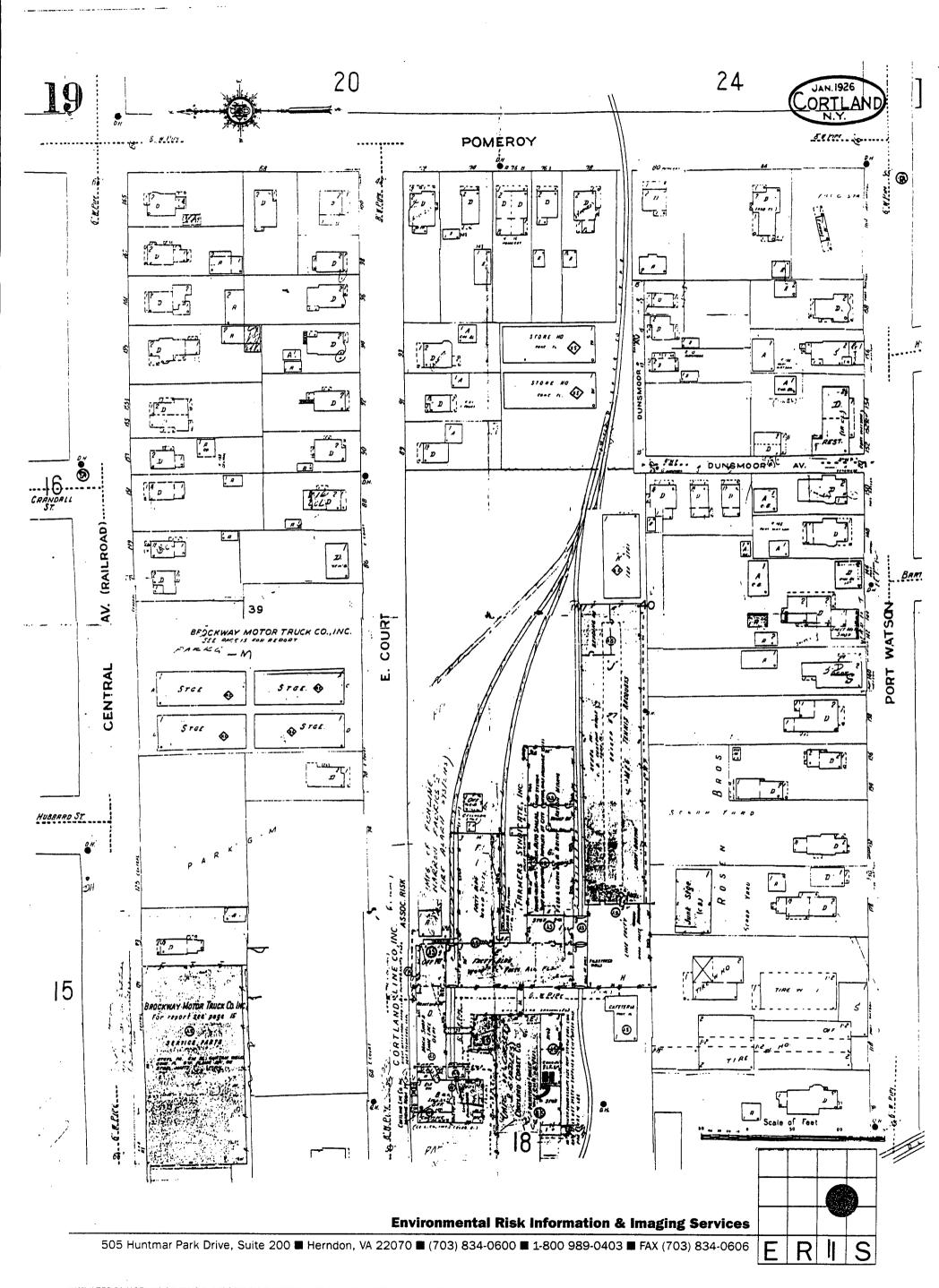
A noncombustible building built in 1962 with concrete block walls; unprotected steel columns and beams; concrete floors on the state of the state of

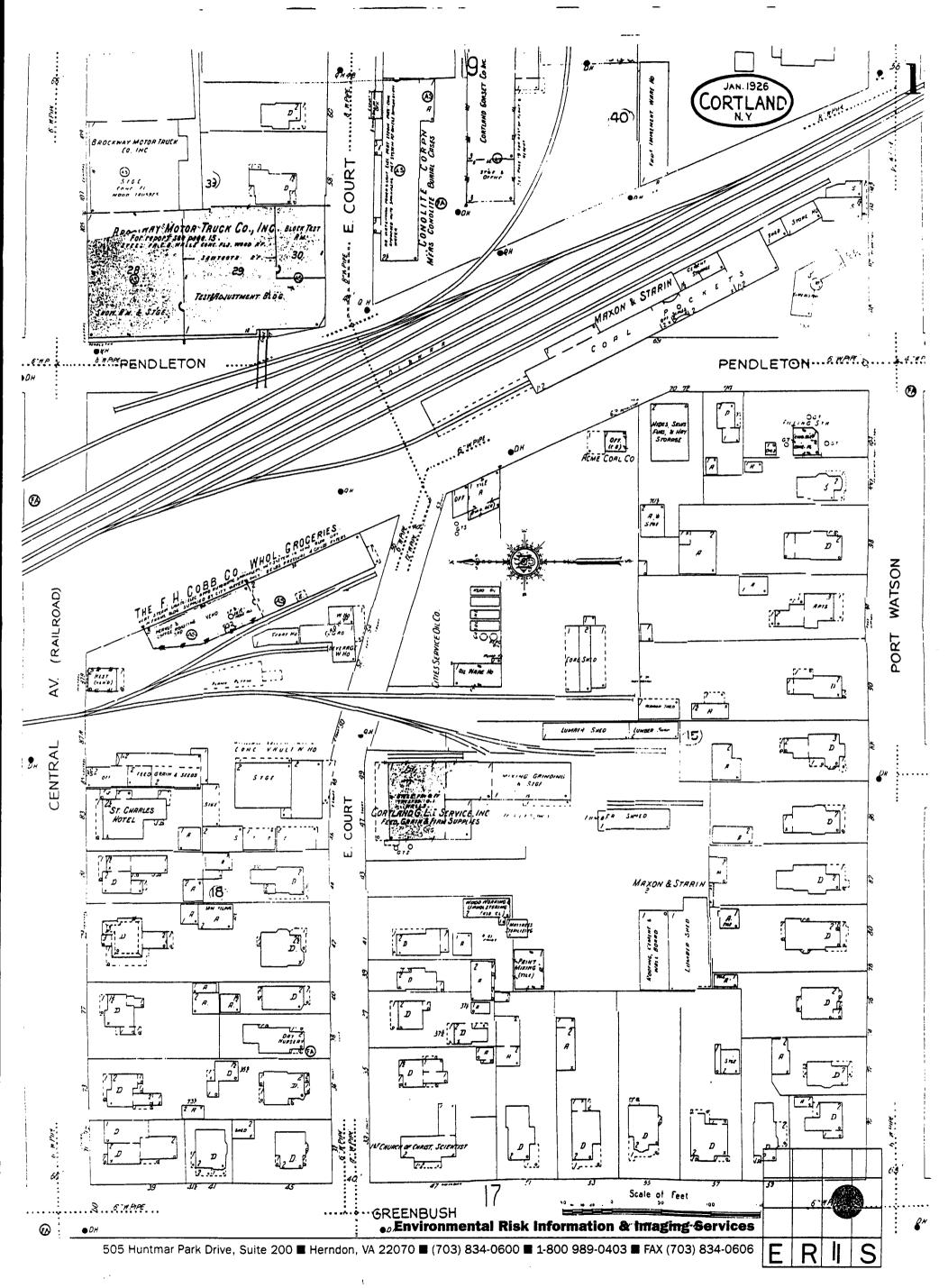


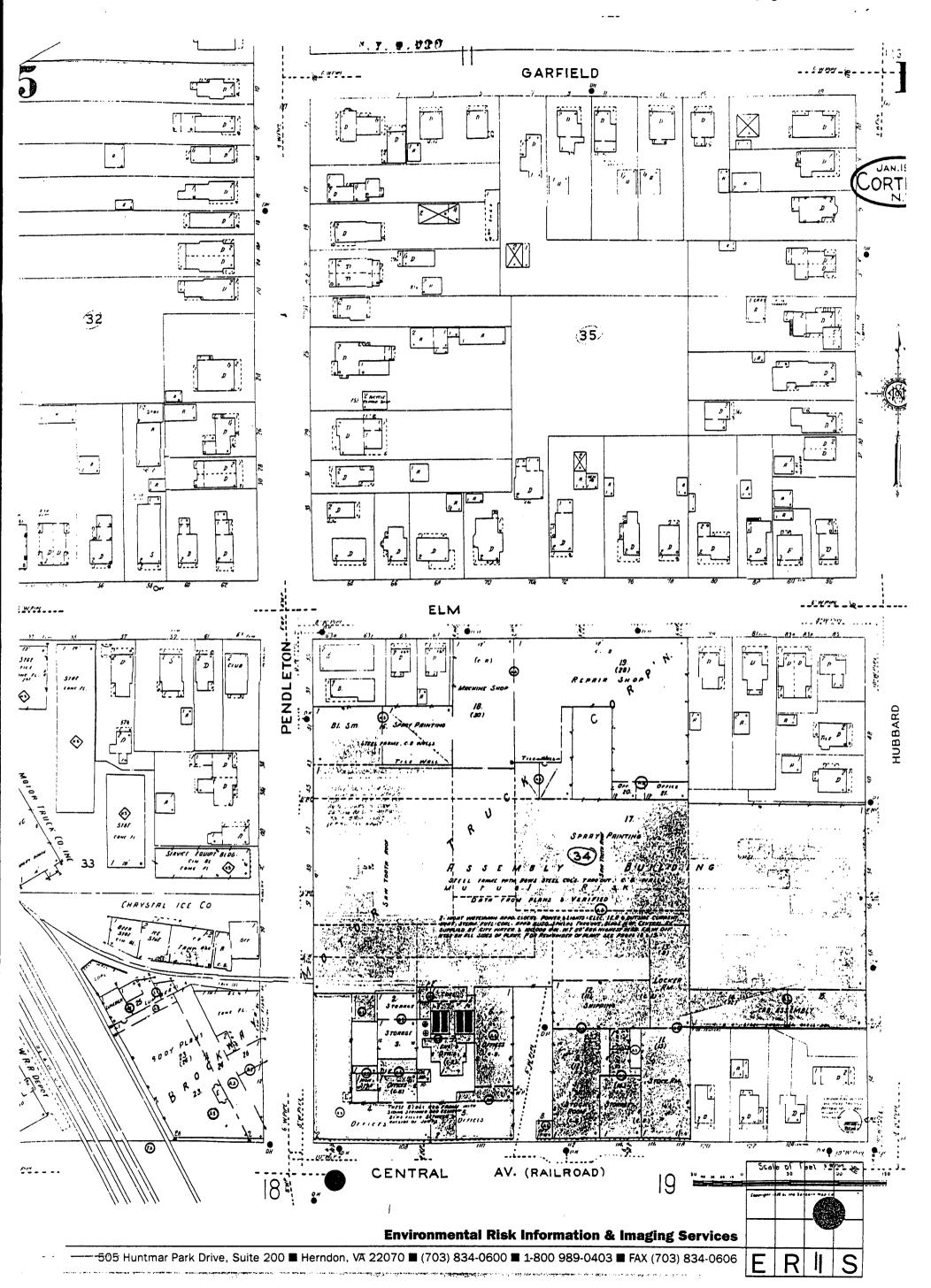


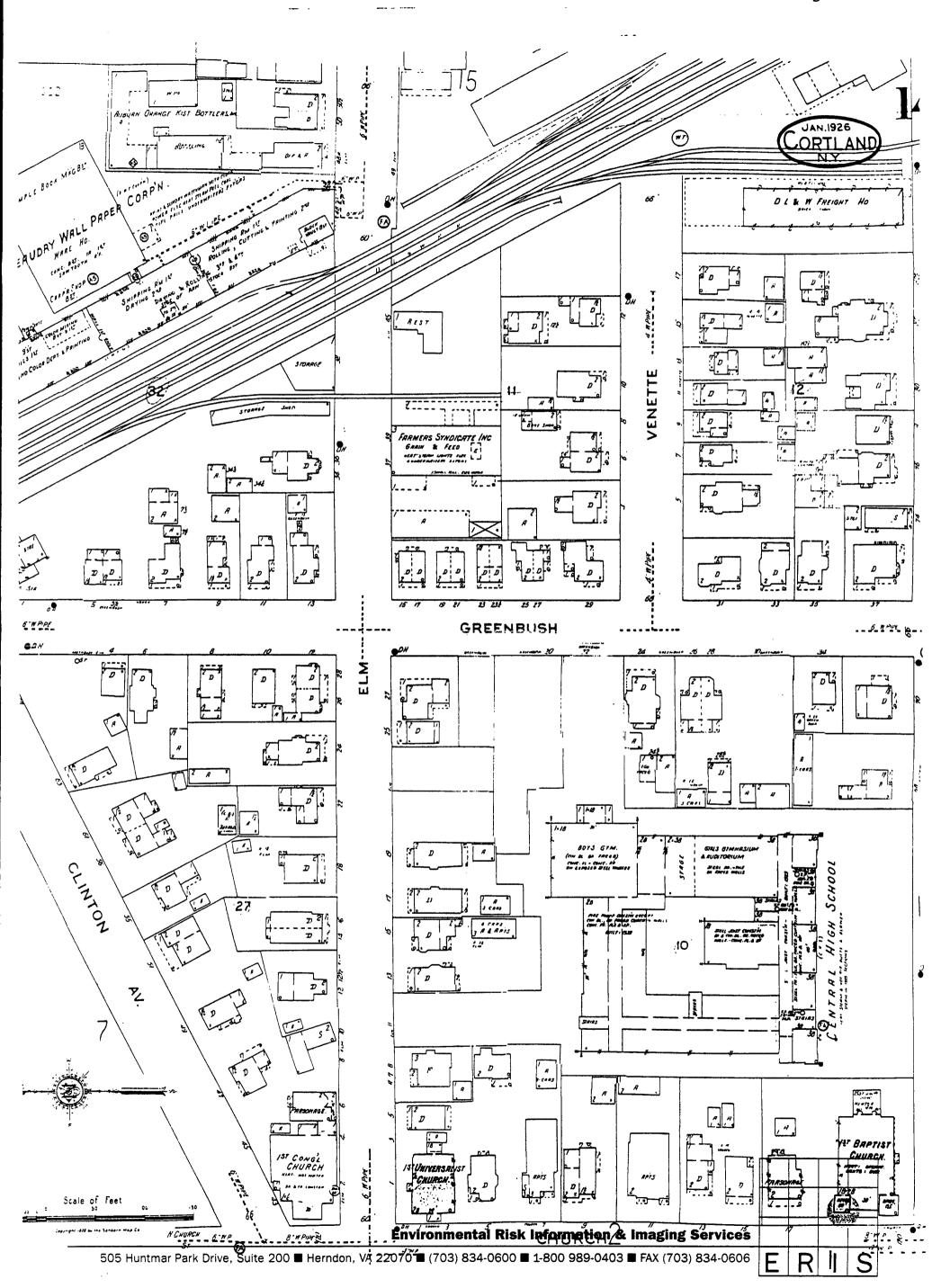
THE REPRODUCTION OF THIS SANBORN FIRE INSURANCE MAP HAS BEEN MADE BY PERMISSION OF SANBORN MAPPING & GEOGRAPHIC INFORMATION SERVICE. THE COPYRIGHT HOLDER, IN ACCORDANCE WITH THE TERMS AND CONDITIONS OF AN AGREEMENT BETWEEN ENVIRONMENTAL RISK INFORMATION & IMAGING SERVICES AND SANBORN MAPPING & GEOGRAPHIC INFORMATION SERVICE DATED AUGUST 1, 1991.

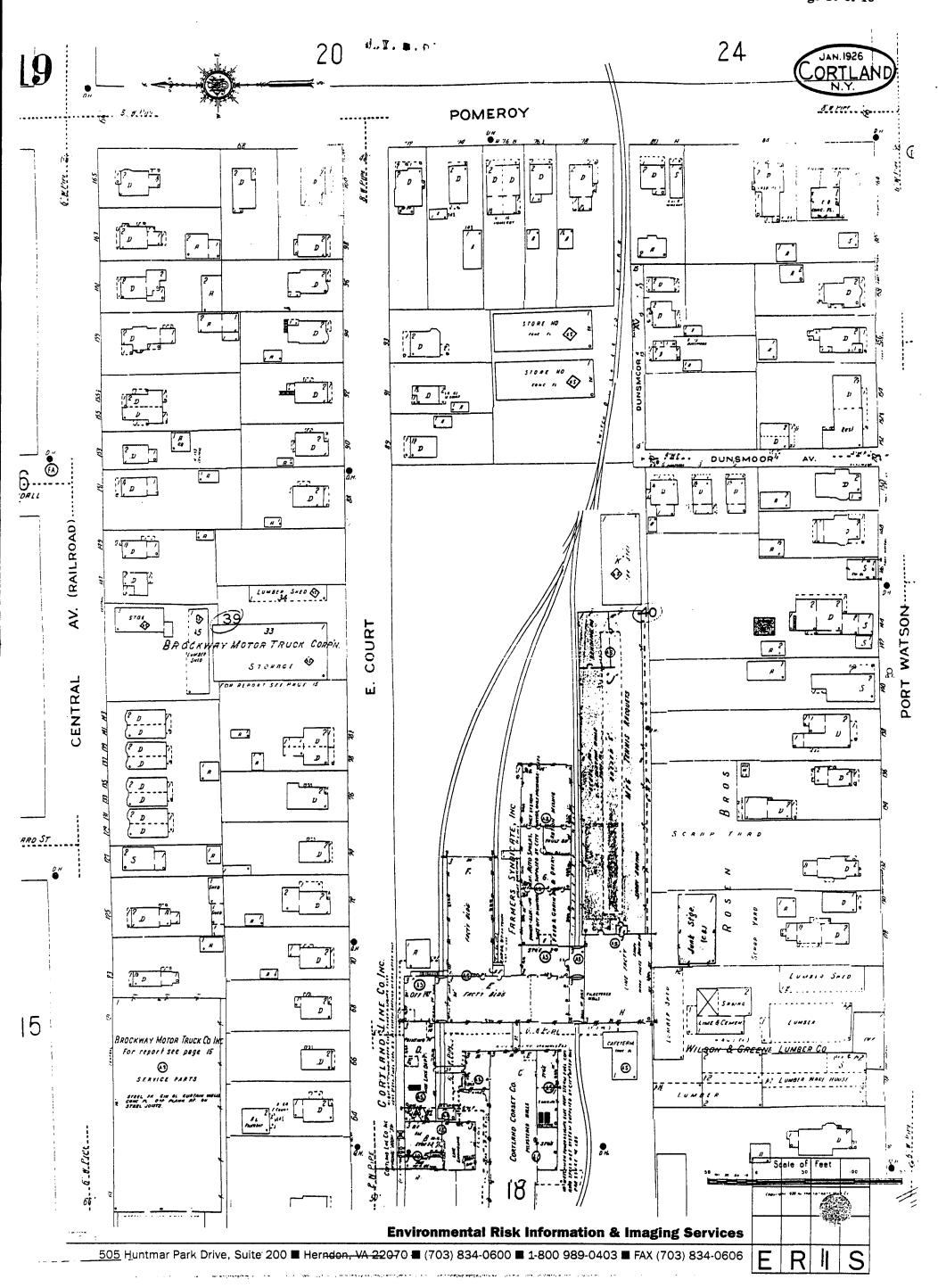


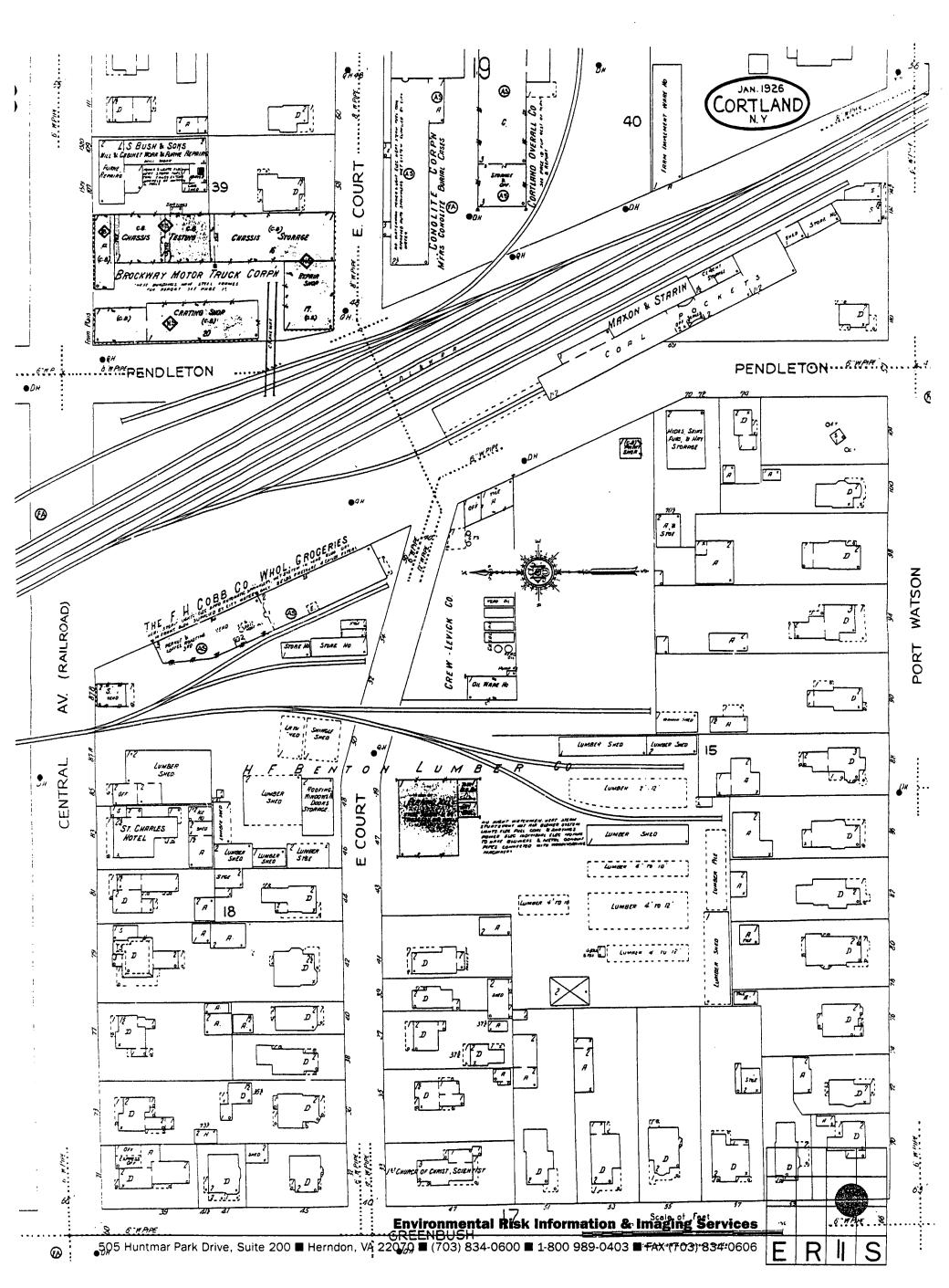












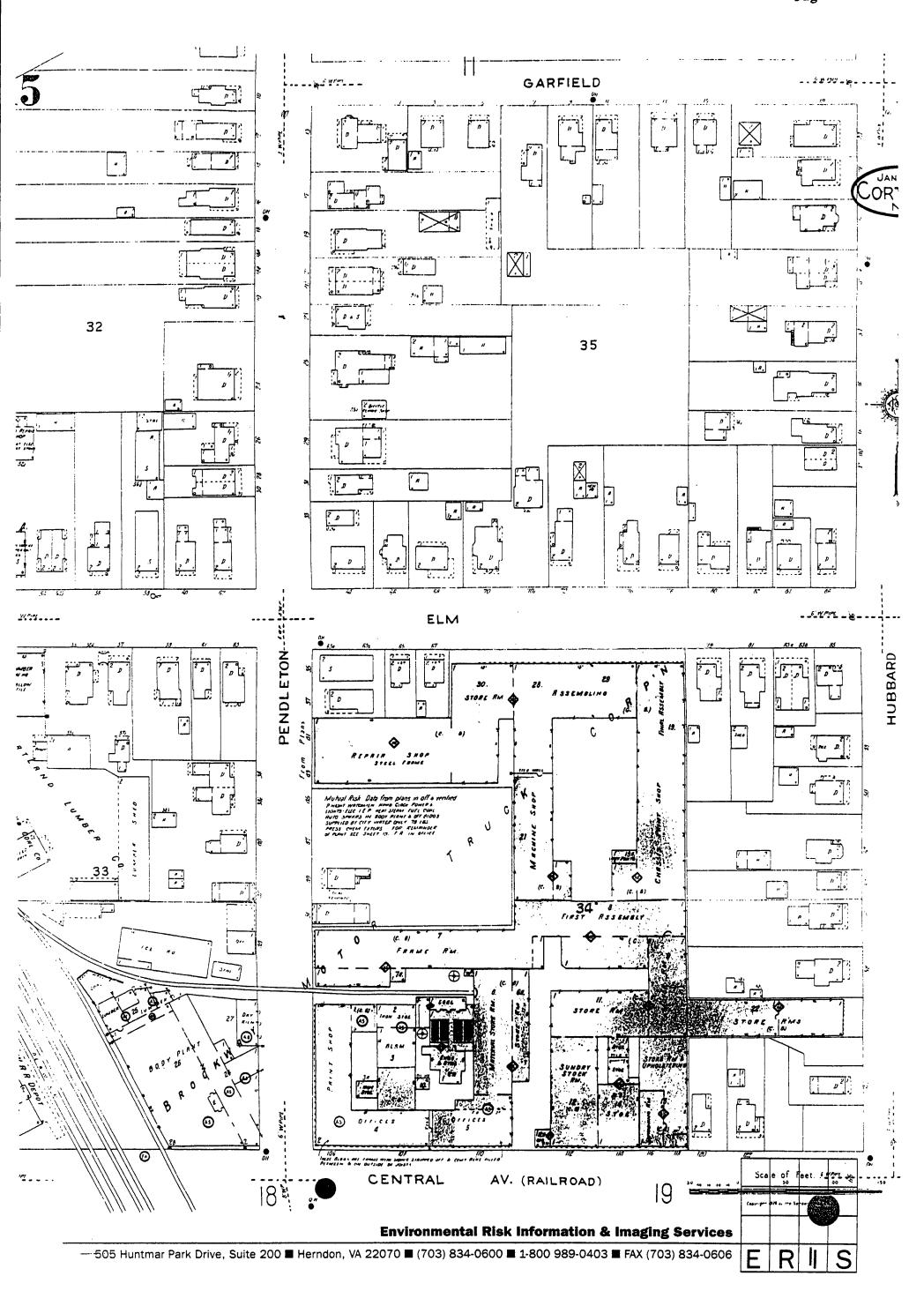
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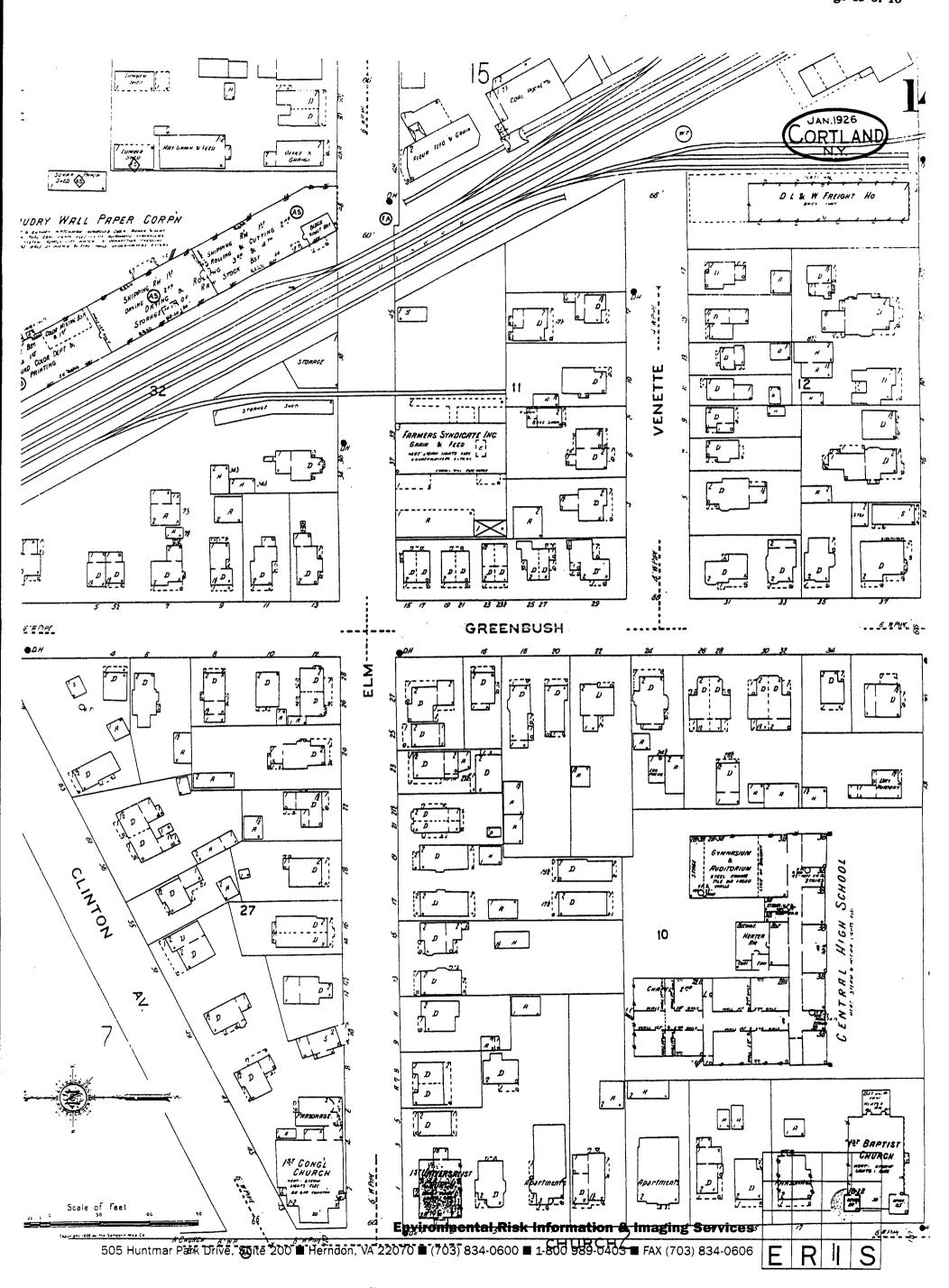
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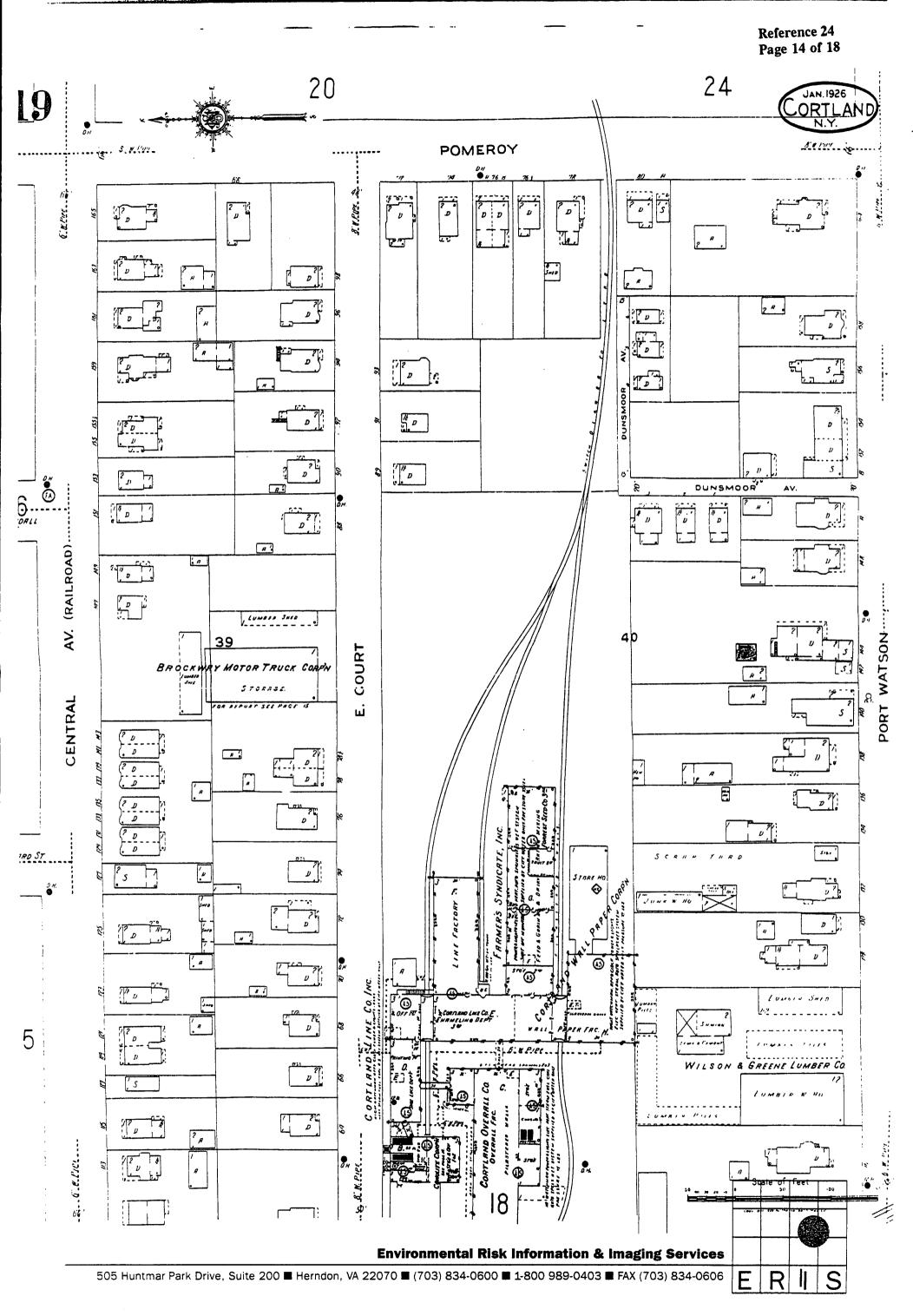
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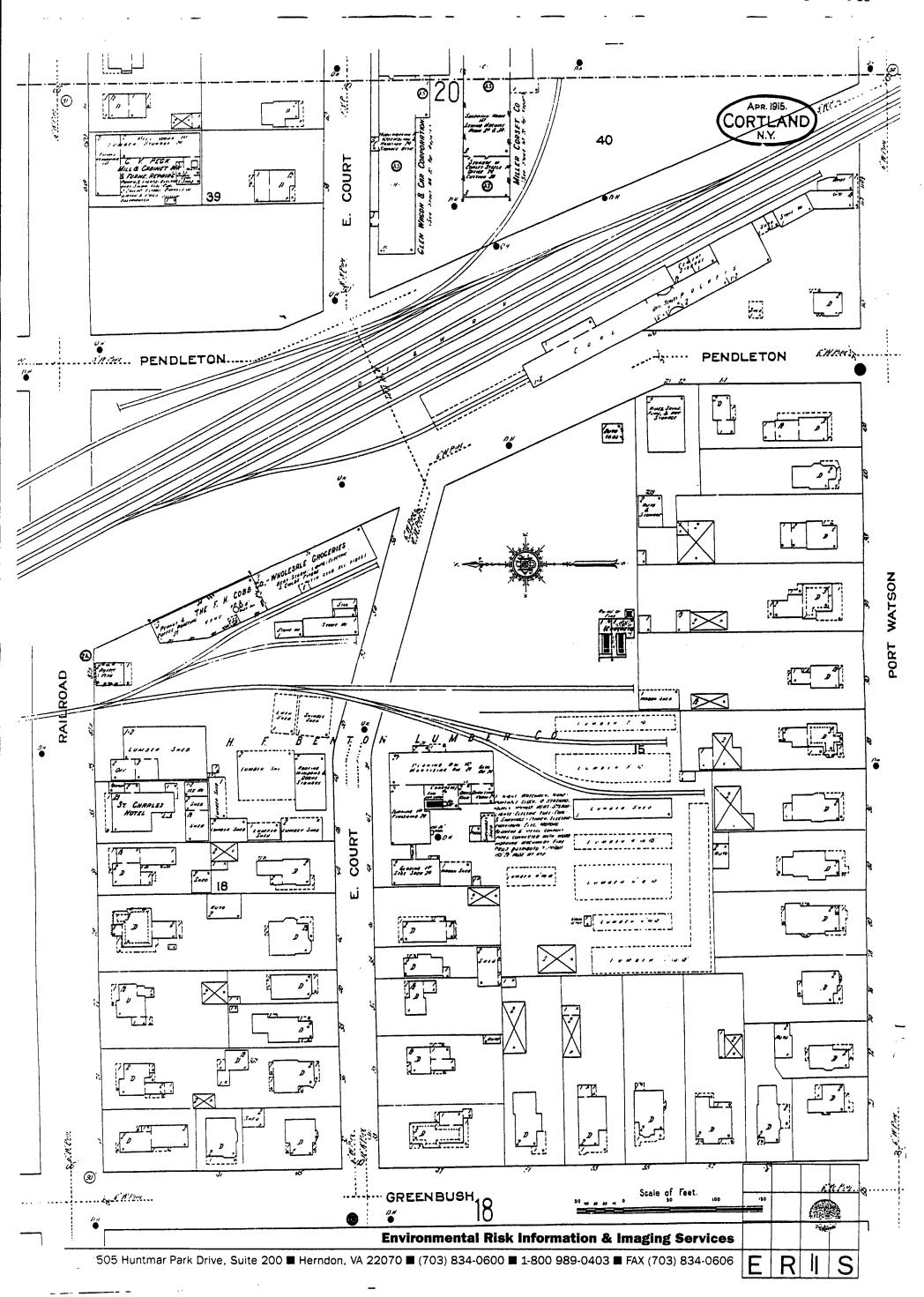
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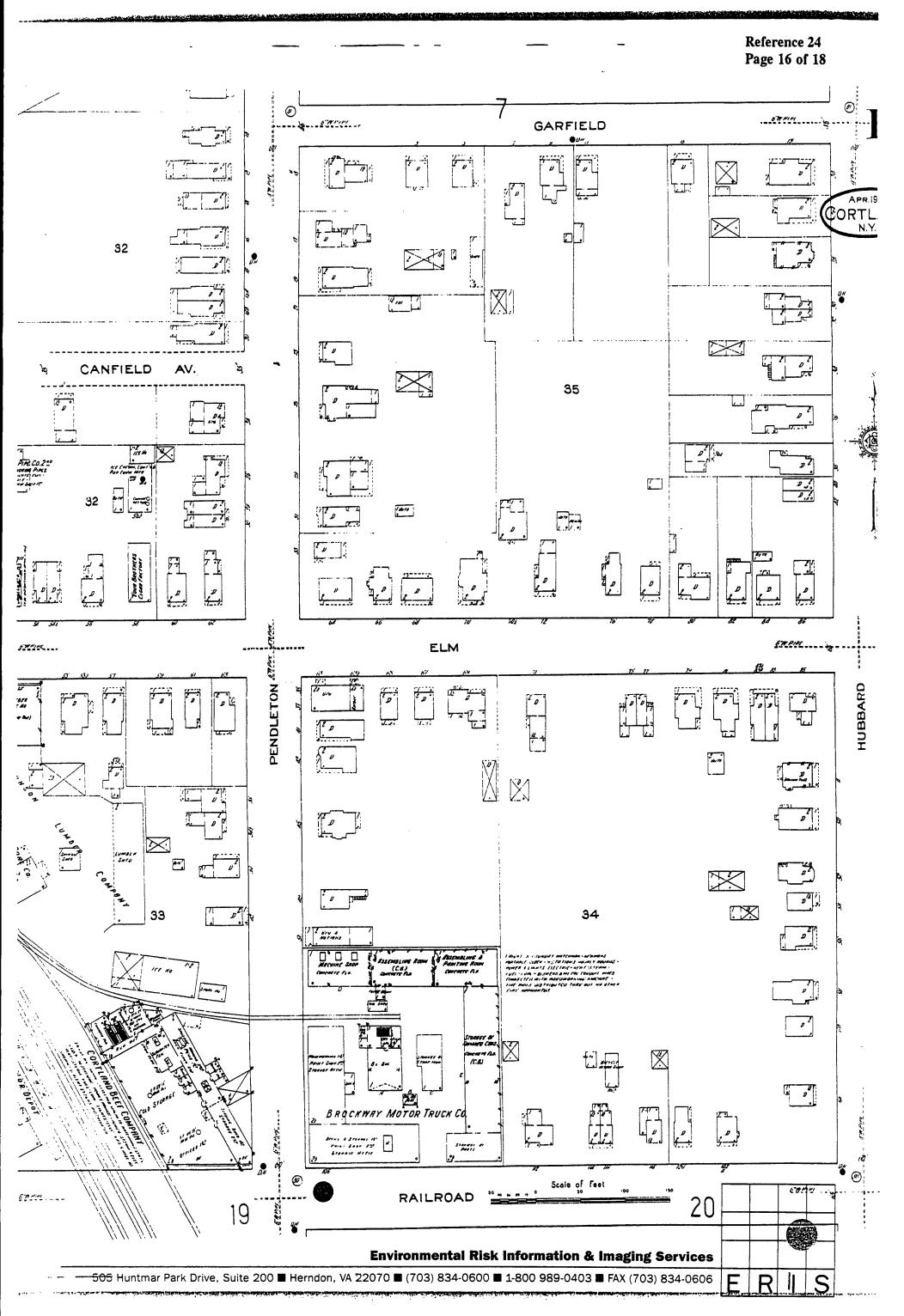
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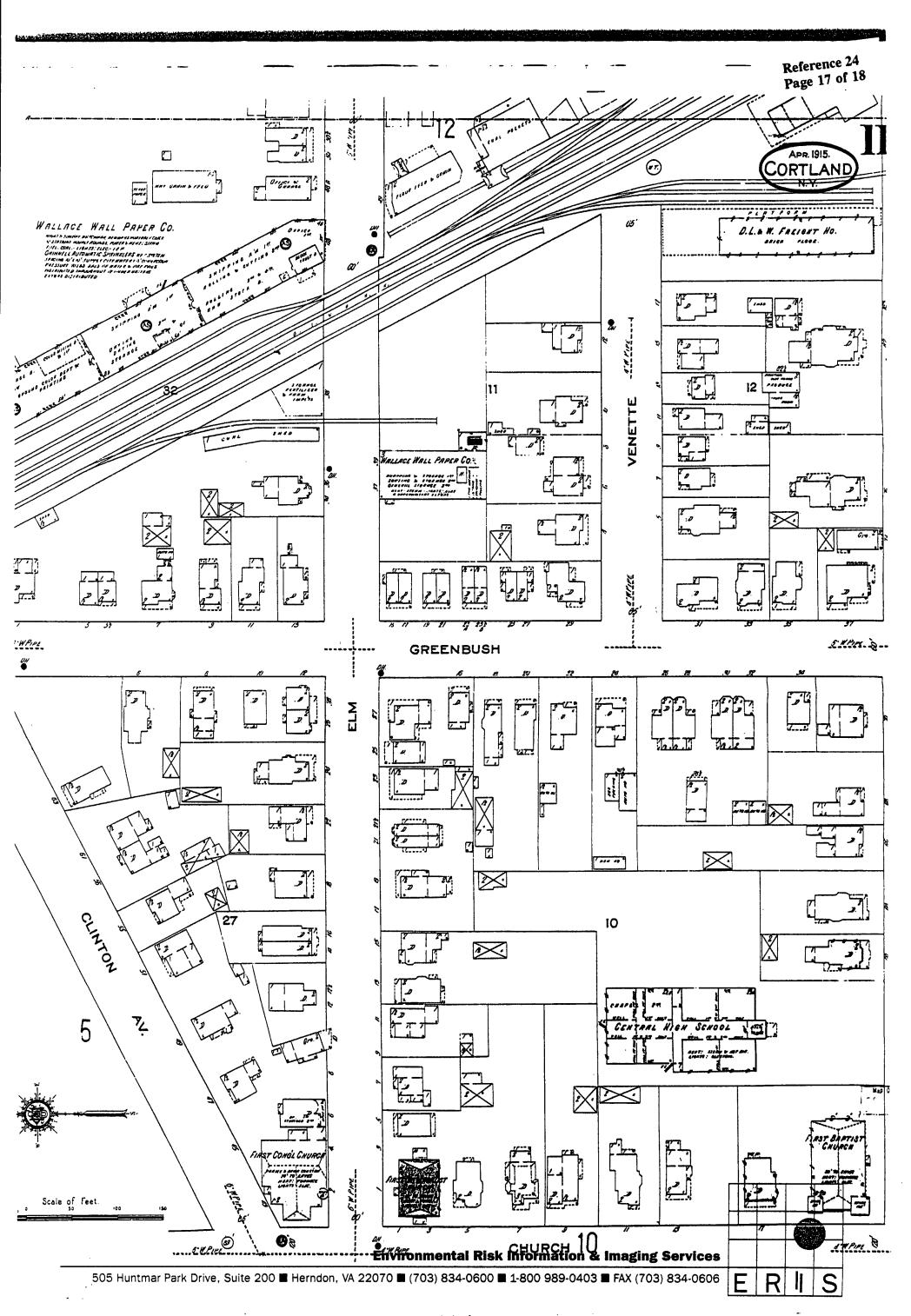


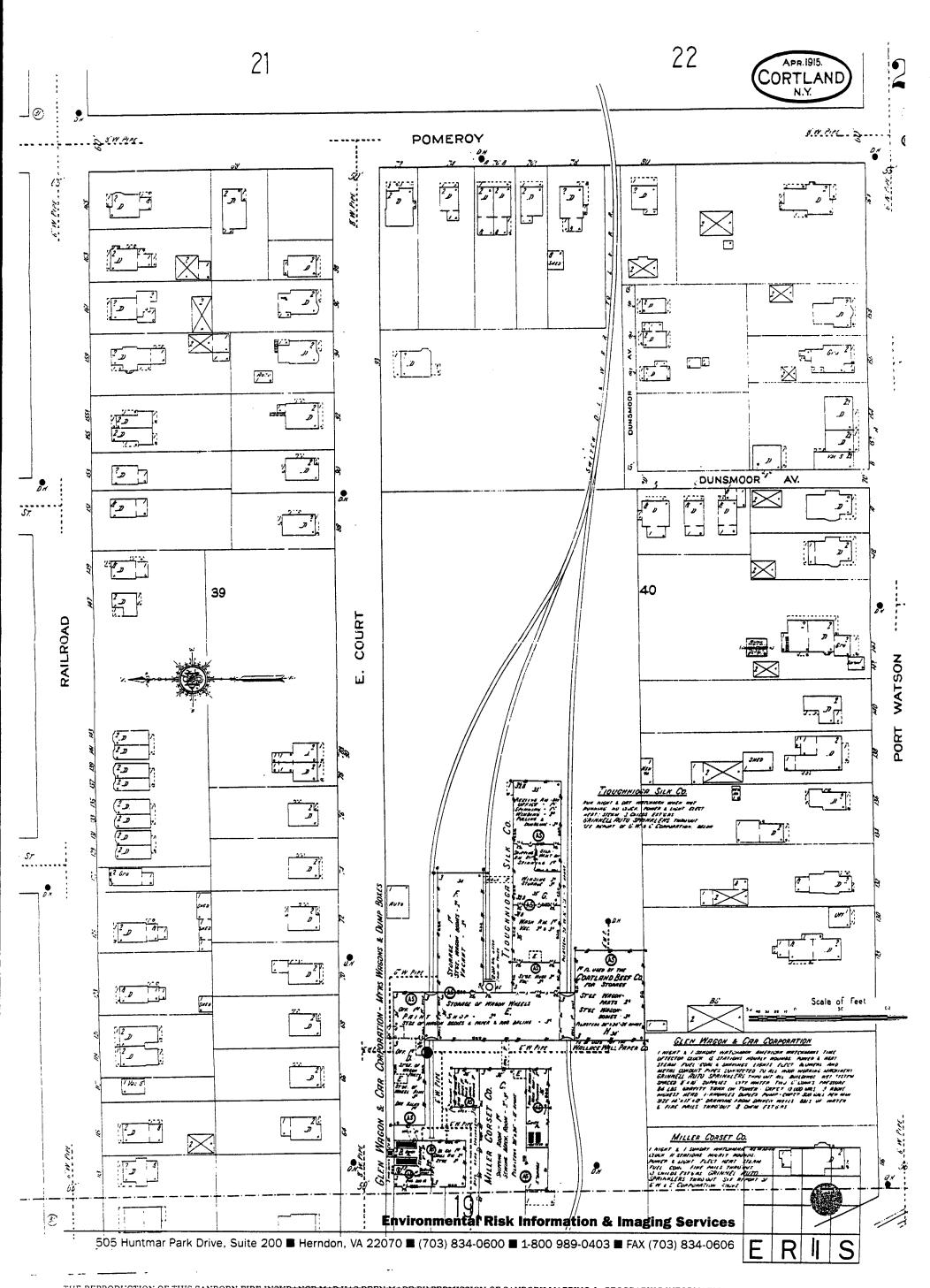












MACK TRUCKS, INC.

Reference 25 Page 1 of 1

WORLD HEADQUARTERS
BOX M
ALLENTOWN, PENNSYLVANIA 18105-5000

Telephone: (610) 709-3011 Telecopier: (610) 709-2186

March 17, 1995

TELECOPY AND U.S. MAIL

Jeffrey L. Martin
Environmental Chemist
Ebasco Environmental
2111 Wilson Boulevard, Suite 435
Arlington, VA 22201-3058

Re: Brockway Motor Trucks in Cortland, N.Y.

Dear Mr. Martin:

As we discussed over the telephone on February 3, 1995, Mack Trucks received your correspondence regarding the request for additional information on an old Mack division plant facility, not owned or occupied by Mack Trucks since the late 1970's. Since that time, Mack Trucks, through its attorney and in-house personnel, has tried to contact the EPA official whose name and number you provided. We have left several messages on Ms. Moyik's phonemail, with no success in talking with her or receiving any return of our messages.

As we discussed, Mack Trucks is in no position to turn over any information without a confirmation of your status as well as that of the EPA's. We will continue to attempt contact with the EPA, but until then, we are left with no other alternative. We trust you appreciate our position.

As soon as satisfactory confirmation is made, we will contact you.

Very truly yours,

MACK TRUCKS, INC.

homas R. Wiles D. T. R. Wilcox, III

Attorney





CANFORD MANUFACTURING CORPORATION

Wholly Owned Subsidiary of Richermaid Incorporated

106 Central Avenue S Cortland, N.Y. 13045-2755 (607) 753-3305

July 23, 1987

Mr. John A. Ducar NUS Corporation 1090 King Georges Post Road Edison, NJ 08837

SUBJECT: Additional Requested Information for Site Inspection Report for Brockway Motor Truck EPA NY 980203111 Survey

Dear Mr. Ducar;

Per your letter dated July 13, 1987, to Mr. Ron Smith I'm attaching the additional information that you requested.

Please feel free to contact me if you need additional information concerning this matter.

Sincerely,

William G. Lewis

Facilities Engineer

WGL/dap

Attachment

- 1. Current employment at Rubbermaid-Cortland Inc. Facility as of July 15, 1987, is 295.
- 2. Rubbermaid-Cortland Inc. (An Affiliate of Rubbermaid Incorporated) U.S. EPA Number is NYD 057025777 Issued: January 21, 1983 Expiration: Permanent Permit

Rubbermaid-Cortland Inc. Bulk Storage Facility Permit #810
Issued: January 21, 1987
Expiration: Permanent Permit

Storage/Disposal

Store approximately 110 gallon/mo. in 55 gallon drums. Disposal of approximately 110 gallons/mo. or 1,110 lbs. in 55 gallon drums.

Materials: 111, Trichloroethane Waste # *F002 UN 2831 Materials: Petroleum Naphtha Waste # *D001 UN 1255

*Recycle by scavengers

- 4. Name of transporters and scavengers:
 - A. Solvents & Petroleum Services 1405 Brewerton Rd. Syracuse, NY 13208
 - B. Safety-Kleen Corporation Factory & Mitchell Matydale, NY 13211
 - C. Environmental Oil Inc. P.O. Box 315 Syracuse, NY 13209
- 5. Name of previous owner of properties prior to Rubbermaid's acquisition:
 - a) North side of Central Avenue between Pendleton Street and Hubbard Street.

Canford Manufacturing Corporation, a subsidiary of Stanhome Inc.

(This property is being leased to Rubbermaid-Cortland Inc. by the owner, the Cortland County Industrial Development Agency.)

b) North side of Central Avenue between Pendleton Street and Greenbush Street.

New York, Susquehanna and Western Railway Corporation

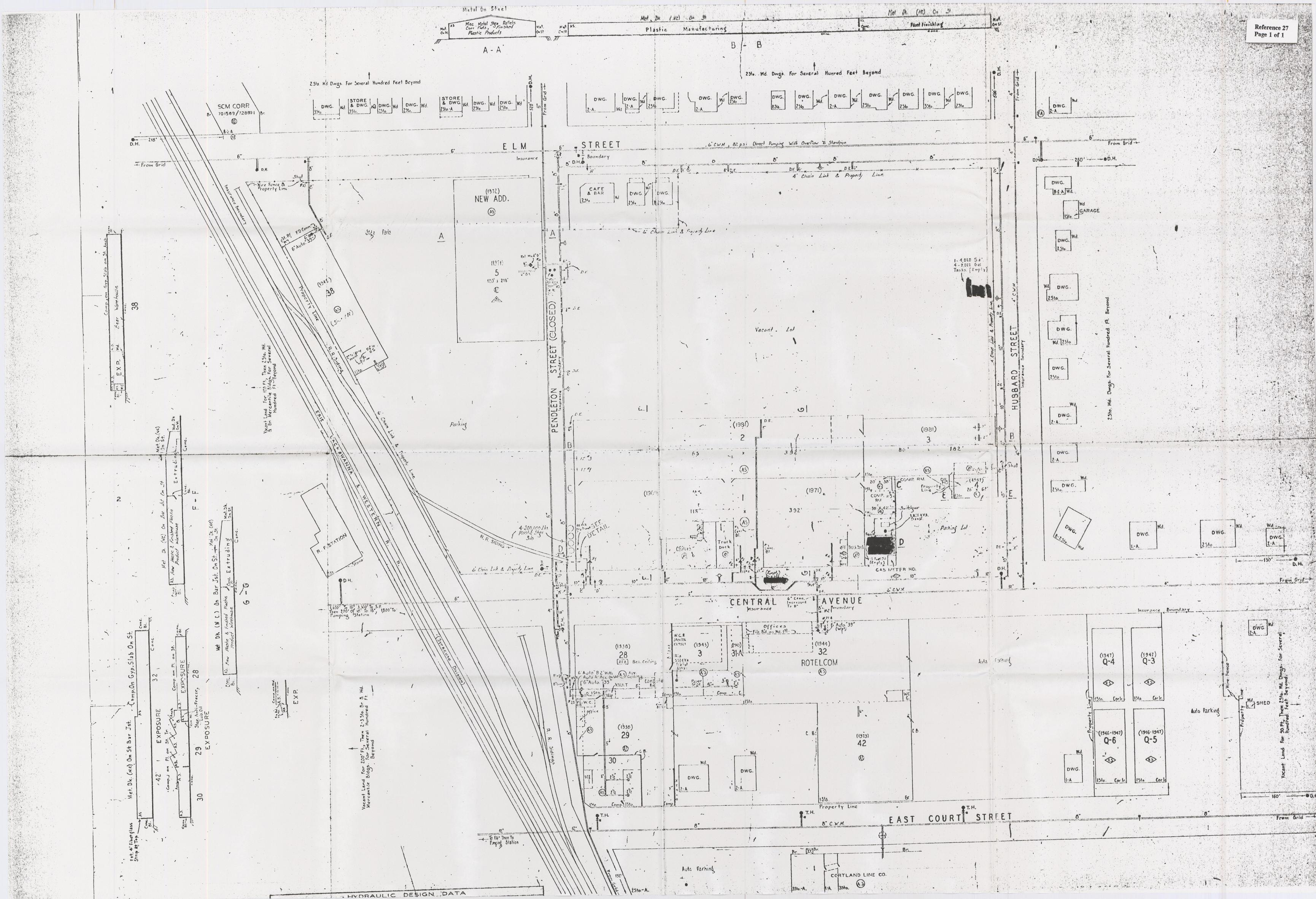
c) North side of Central Avenue at the corner of Pendleton Street and Elm Street.

Paul A. Sepe and Georgianna T. Sepe.

d) South side of Central Avenue between East Court Street and Pomeroy Street.

Joseph Compagni, Jane Compagni and Joseph H. Compagni.

- 6. Number of tanks removed from site since Rubbermaid's acquisition.
 - 7 tanks removed on or about mid-July 1985
 - 1 tank removed on or about early October 1986
 - 1 tank permanently closed on or about the end of Dec. 1986
 - 9 total tanks removed or permanently closed since Rubbermaid's acquisition.



EBASCO SERVICE	= Reférence 28 p. lof (
PROJECT: Brackway Motor Trucks		TELECON NOTE
PROJECT NO. 8310.0076.0000.50042	DATE: 6/22/95	TIME: 1520
DISTRIBUTION: File	•	
BETWEEN Peter Rynkiewitz	OF: Cortland County Health Depl.	PHONE: (607) 753-5035
AND: Jeff Martin MM	-	
DISCUSSION:		
Summary of Convesation		
Peter indicates there	s No well head !	Protection
areas for the local		
are working on est.	ablishing some	
	•	
Peter indicated the a	coundwater is kno	own to
be used for the u	rater and processing	of
commercial diary co		
is not known to be	e used as a source	e of
water, except for	Binington, 2 30-	Homiles
downstream.		
	•	
		•
	**	
ACTION ITEMS:		
EBASCO		

POTENTIAL HAZARDOUS WASTE SITE PRELIMINARY ASSESSMENT

IDENTI OI STATE 02

Page 1 of 2

PART 1 - SITE LOCATION AND INSPECTION INFORMATION D980203111 NY TTE NAME AND LOCATION SITE NAME (Legal, common, or descriptive name of site) O2 STREET, ROUTE NO., OR SPECIFIC LOCATION IDENTIFIER Brockway Motor Trucks 106 Central Avenue 05 ZIP CODE 06 COUNTY 07 COUNTY 04 STATE CITY 08 CONG DIST. CODE Cortland 13045 Contland 023 25 COORDINATES LATITUDE LONGITUDE 4 2º 3 5" 3 0'. N _0 7 _60 _1 _0" 3 0'. W 10 DIRECTIONS TO SITE (Starting from nearest public road) erstate 81 to Cortland. Use Exit 11. Proceed east, past Holiday Inn, to third light (Central Avenue). Right on Central ut % mile to plant. III. RESPONSIBLE PARTIES OWNER (if known) O2 STREET (Business, mailing, residential) Cortland County Industrial Development Agency 50 Main Street 03 CITY OF STATE 05 ZIP CODE 06 TELEPHONE NUMBER Cortland 13045 OPERATOR (if known and different from owner) 08 STREET (Business, mailing, residential) Rubbermaid, Inc. 106 Central Avenue CITY 10 STATE 11 ZIP CODE 12 TELEPHONE NUMBER Cortland 13045 (607) 753-3305 TYPE OF OWNERSHIP (Check one) B. FEDERAL: A. PRIVATE C. STATE X D. COUNTY E. MUNICIPAL (Agency name) F. OTHER: G. UNKNOWN (Specify) OWNER/OPERATOR NOTIFICATION ON FILE (Check all that apply) A. RCRA 3001 DATE RECEIVED: _ / / X B. UNCONTROLLED WASTE SITE (CERCLA 103 c) DATE RECEIVED: 06/08/81 C. NONE CHARACTERIZATION OF POTENTIAL HAZARD ON SITE INSPECTION BY (Check all that apply) YES DATE: 12 / 16 / 82 A. EPA _ B. EPA CONTRACTOR _X C. STATE D. OTHER CONTRACTOR X E. LOCAL HEALTH OFFICIAL F. OTHER: (Specify) CONTRACTOR NAME(S): SITE STATUS (Check one) **03 YEARS OF OPERATION** X A. ACTIVE B. INACTIVE C. UNKNOWN 1969 Present UNKNOWN BEGINNING ENDING DESCRIPTION OF SUBSTANCES POSSIBLY PRESENT, KNOWN, OR ALLEGED presence of inorganics, solvents, heavy metals, acids and bases are possible, due to their use at the plant in the past. 05_DESCRIPTION OF POTENTIAL HAZARD TO ENVIRONMENT AND/OR POPULATION tential hazard exists to surface water, groundwater, and soil contamination. The population in the area uses groundwater drinking. IV. PRIORITY ASSESSMENT RIORITY FOR INSPECTION (Check one. If high or medium is checked, complete Part 2 - Waste information and Part 3 ription of Hazardous Conditions and Incidents) A. HIGH B. MEDIUM C. LOW D. NONE (Inspection required promptly) (Inspection required) (Inspection on time available basis) (No further action needed. complete current disposition form) INFORMATION AVAILABLE FROM 01 CONTACT 02 OF (Agency/Organization) 03 TELEPHONE NUMBER iana Messina U.S. EPA Region II, Edison, NJ (201) 321-6776 04 PERSON RESPONSIBLE FOR ASSESSMENT 05 AGENCY 06 ORGANIZATION OF TELEPHONE NUMBER OS DATE ohn Ducar

EPA

FORM 2070-12 (7-81)

NUS FIT II

(201) 225-6160

02 / 24/ 87



02-8701-22-PA

POTENTIAL HAZARDOUS WASTE SITE

PRELIMINARY ASSESSMENT

	PRELIMINARY A	SSESSMENT CONTRACTOR
	Brockway Motor Trucks Site Name	NYD980203111 EPA Site ID Number
	106 Central Avenue Cortland, New York Address	02-8701-22 TDD Number
-	Date of Site Visit: 02/04/37	
	SITE DESCRIPTION	
	Brockway Motor Trucks is a 22-acre single. and used as a truck assembly planesidential area at 106 Central Aven former factory building is currently Industrial Development Agency and imanufacture and distribute plastic procacross the street is owned by Canford tanks containing solvents from the prop There is still an unknown number of distribute plastic procacross the street is owned by Canford tanks containing solvents from the prop There is still an unknown number of distribute plastic procacross the street is owned by Canford tanks containing solvents from the prop There is still an unknown number of distribute plastic planes.	ant. The facility is located in a nue in Cortland, New York. The owned by the Cortland County is leased by Rubbermaid, Inc. to ducts. The former office building in Mfg. The NYDEC removed two erty now occupied by Rubbermaid.
	PRIORITY FOR FURTHER ACTION: HI	gh Medium X Low
	A site inspection is recommended. The groundwater, surface water, soil and the sampling.	he potential for contamination of se sanitary sewer system warrants
	·	
	Prepared by: John A. Ducar Date of NUS Corporation	te: _02/24/87

02-8704-17-SR Rev. No. 0

SUMMARY STATEMENT BROCKWAY MOTOR TRUCKS

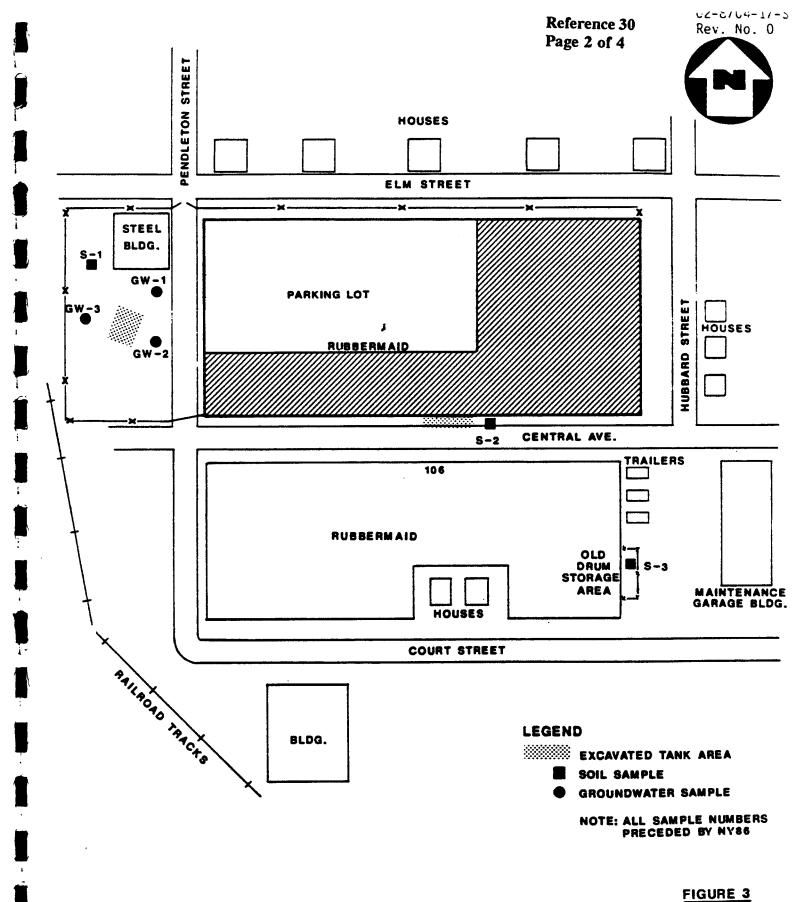
Brockway Motor Trucks is a 20.5-acre site that was owned by Mack Trucks, Inc. and used as a truck assembly plant from 1969 to 1977. The facility is located in a moderately populated commercial/residential area on Central Avenue in Cortland, New York. The former truck factory building is currently owned by the Cortland County Industrial Development Agency and leased by Rubbermaid Inc. (1983 - Present) to manufacture and distribute plastic products.

In February 1987, a 55-gallon drum of 1,1,1-trichloroethane (TCE) was crushed by a forklift, allowing approximately 35 gallons of the solvent to spill onto the ground. The NYSDEC was at the site the same day to oversee the excavation and removal of the contaminate soil. Soil analysis showed the presence of 1,1,1-TCE.

The primary concern at this site is the potential contamination of the drinking water aquifer. Residents within the city limits of Cortland obtain their drinking water from a municipal supply system. The municipal water supply is obtained from two wells located west of the site in the city of Cortland. The potential population affected within a 3-mile radius is 24,851.

This report will not deal with formerly buried diesel and gasoline fuel tanks because they are not covered under CERCLA. It is recommended that further investigation be conducted in this area.

Results of sampling at the site showed lead in concentrations over five times greater in the downgradient well than in the upgradient well. Aluminum, barium, chromium, copper, magnesium, and vanadium were also detected in concentrations significally greater in the downgradient well.



SAMPLE LOCATION MAP

BROCKWAY MOTOR TRUCKS, CORTLAND, N.Y.





BROCKWAY MOTOR TRUCKS, CORTLAND, NEW YORK



1P-7 July 8, 1987
J. Murtaugh and T. Varner taking sample GW-3.
Photographer: John Ducar.



July 8, 1987 J. Murtaugh taking sam**p**le S-1. Photographer: John Ducar.



BROCKWAY MOTOR TRUCKS, CORTLAND, NEW YORK

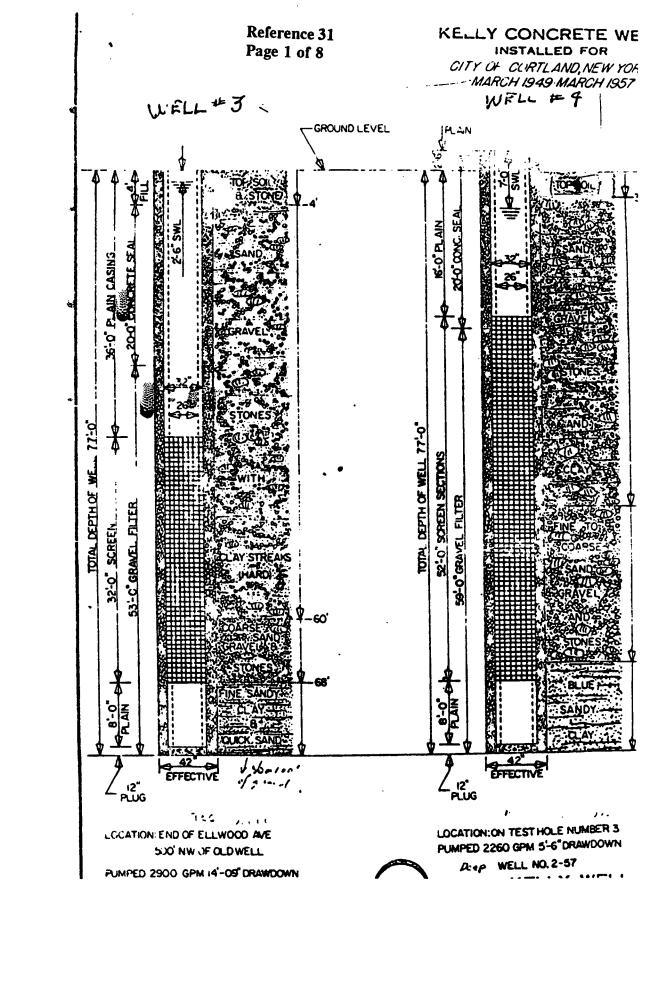


1P-9 July 8, 1987
T. Varner taking sample S-2.
Photographer: John Ducar.

1630



July 8, 1987
T. Varner taking sample S-3.
Photographer: John Ducar.



WELL LCG						
	WET	π.	٠.	Τ.	O	G

ell No3 (Hall #1)		Starting Date 21 Completed 12 Sep	
Tob name and address	Town of Cortlandvil	lle	
etailed location	i e production de la company		
		en en de la companya	
ngineer Rowell Associa	tes Report to		
rilling Instructions	As per Contract		
tatic Well Dia	o-6"	oth <u>Material</u> Topsoil	•
rc	6"-30"	Dry Gravel, San	d &
	30-63'	Boulders Coarse Sand & G	ravel,
	63-65'	Water-bearing S	
	65-701	Gravel with Cla Glacial Till, d no water	
	Left-in-pl	ace, 50' of 12" welded casi	ing
		15' of 12" Johnson tel	escopin
		S/S well screen, 100 lead packer top, clo) slot,
		bottom	<u> </u>
	Pumped 26	hours @ 660 GPM, pumping le	evel 45
	BTC. Casi	ing terminated 12" above gra	ade.
19	Top of sci	reen at 49' below grade(acti	ual ·
		of bottom of 12" casing).	
	_		,
Screen data per aborceen Size	ove notes. Setting	Exposure	,
	1	R. Rowe	•

Invoice No.

McLean, New York Owner's Name Town of Cortlandville Address Lime Hollow Rd. (site) Cortland, NY Size of Well 6 inch Contract No. Mach. No. Driller Type G.P.M. Draw Down Total Depth in Feet Date Started 12-29-87 Date Completed 2-16-88 CASING ROCK GASOLINE FORMATION AND REMARKS IN FEET DRILLED IN GALS. 69ft. - Gravel (some fine sand) 70 - 71 - Gravel (mostly Sand) 72 - hard packed sand- (fine & coarse stones) 73 - Sand (some stones) 74 - Gravel & Sand (some fine) 75 - Coarse Sand & Gravel 75 - bailed 20 gpm 76 - Coarse Sand & Gravel 77 - Coarse Snad (some gravel) 78 - Sand (some hardpan on bit) 79 - 80 - Sand -(fine to coarse - some stones) 81 - Sand (fine to coarse) 82 - Fine Shad & Gravel 83 - Gravel & Sand 84 - 85 - Gravel 85 - 86 -Gravel & Sand 86-90 - Gravel & Coarse Sand 90 - 91 - Quck Sand & Clay

REMARKS:

.III

DRILLER'S LOG

RANDOLPH WELL & PUMP CO., INC.

McLean, N. Y

Invoice No.____

Owner's Name Town of Cortlandville				Address L	ime Hollow	Rd., Cortland, N.	Υ.			
Contract N	Contract No. Size of Well 6 in				No.	Driller				
Туре	V	G.1	Р.М.	Draw Dow	Draw Down Total Depth in Feet					
Date Star	ted	2-	9-88	Date Comp	oleted 2-10-					
CASING IN FEET	ROCK DRILLED	GASOLINE IN GALS.		IARKS	1					
				evelopment						
			2-9-88	4½ Hrs.						
		 	2-10-88	8½ Hrs.						
		***************************************				.				
			Static Wat	er Level 32 ft	. 6 inche	8				
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Reference 31
Page 4 of 8

REMARKS:



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SIEVARI EROLL INC

Integral and Municipal Well Water Wests

Doc. 28, jiees

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Figure II

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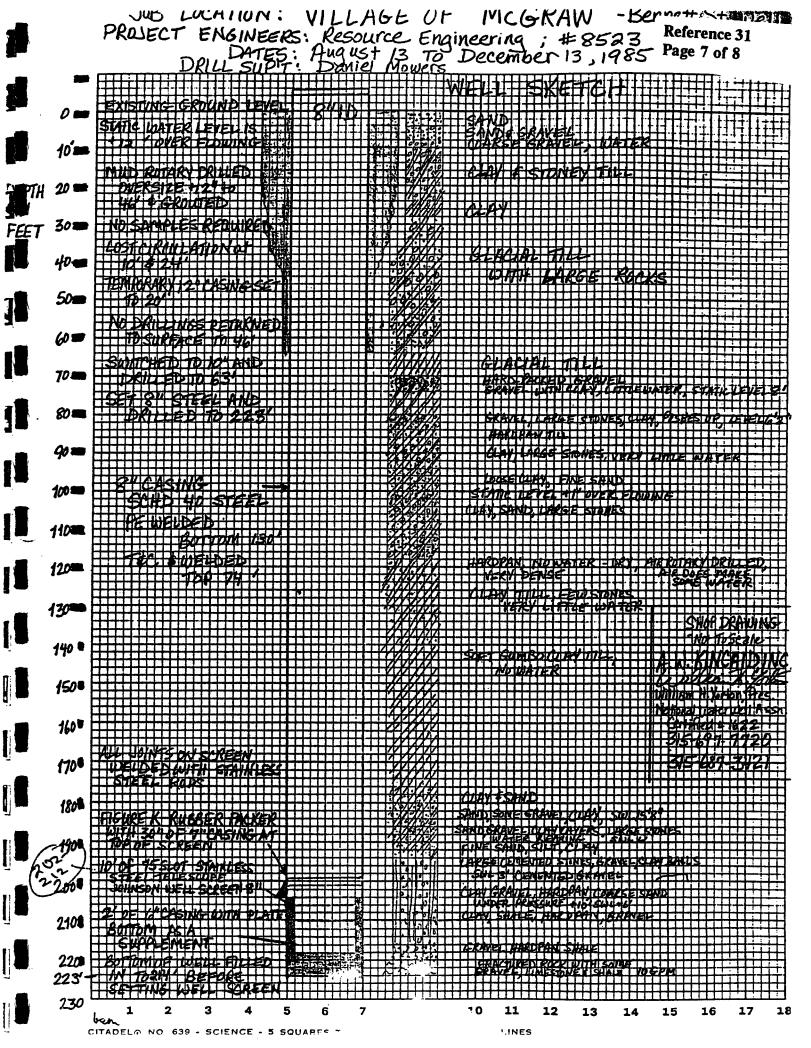
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IESI DUNING LUG

EAST SYRACUSE, N.Y. 13057

PROJECT

Bennett Street Well Site

McGraw, New York

HOLE NO. B-1 Reference 31

LOCATION

SURF. EL.

Page 8 of 8

DATE STARTED

9/1/93

DATE COMPLETED

9/1/93

93248 JOB NO.

GROUND WATER DEPTH WHILE DRILLING 10.0

N — NO. OF BLOWS TO DRIVE SAMPLER 12" W/140# HAMMER FALLING 30" — ASTM D-1586, STANDARD PENETRATION TEST

BEFORE CASING

REMOVED

12.41

C - NO. OF BLOWS TO DRIVE CASING 12" W/ "/OR - % CORE RECOVERY

HAMMER FALLING

AFTER CASING

Hole caved

REMOVED

at 2.5'

CASING TYPE - HOLLOW STEM AUGER

SHEET 1 OF 1

DEPTH	SAMPLE DEPTH	SAMPLE NUMBER	С	SAMPLE DRIVE RECORD PER 6"	N	DESCRIPTION OF MATERIAL	STRATA CHANGE DEPTH
5. 0.							
	5.0'- 7.0'	1		24/20 23/20	43	Brown dry hard SILT, some fine to coarse gravel, little fine to coarse sand	
10.0							10.0
WL	10.01-	2		11/6	·	Brown wet medium dense fine to coarse	
	12.0'	-	·····	6/7	12	SAND, some fine to coarse gravel,	
	12.0'- 14.0'	3		7/10	20	trace silt	14.0'
15.0	14.0'-	4		12/29	- 20	Brown wet very dense fine to coarse	17.0
. 13.0	16.0'			12/7	41		
	16.0'-	5		11/10		little silt	16.0'
	18.01	,		5/4	15	Brown wet medium dense fine to coarse	
	18.01-	6		9/6		SAND and fine to coarse GRAVEL,	
20.0	20.0'	-		9/7	15	trace silt	19.0
	20.0'-	7		7/7		Gray-brown moist stiff SILT, some	
	22.01	ļ		7/7	14	clay, trace fine sand	
						Bottom of Boring	22.0
					 		
25.0							
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Department of Environmental Conservation

Division of Water

New York State Water Quality 1994

Submitted Pursuant to Section 305(b) of the Federal Clean Water Act

June 1994



New York State Department of Environmental Conservation
MARIO M. CUOMO, Governor
LANGDON MARSH, Commissioner

Construction
Agriculture
Agriculture
Land Disposal
Hydromodification

Municipal
Agriculture
CSO's
Agriculture
Agriculture
Agriculture

On-site Systems
Agriculture
Agriculture
Agriculture
On-site Systems
Agriculture
On-site Systems
On-site Systems
Streambank Brosion

Agriculture
Streambank Brosion
Roadbank Brosion
Agriculture
Agriculture
Construction
Municipal
Construction
On-site Systems

Agriculture Source Unknown 'Municipal

SEGMENT			SEGHENT	SECHENT		PRIMARY		PRIMARY	PRIMARY		
NAME	SEG ID	COUNTY	TYPE	SIZE	CLASS	USE IMPAIRED	SEVERITY	POLLUTANT	SOURCE		

DRAINAGE BASIN: Susquehanna River
Sub-Basin: Chenango-Tioughnioga Rivers

BALLYHACK CREEK	0602-0034	Broome	River	3.0	Mi.	C	Pish Propagation	Strenned	Silt (Sediment)
BRAKEL CREEK	0602-0046	Cortland	River	2.0	Mi.	C, CT	Fish Propagation	Stressed	Silt (Sediment)
BRAKEL CREEK	0602-0049	Chenango	River	8.0	Mi.	C, C(T)	Fish Propagation	Stressed	silt (Sediment)
BROOKS CREEK	0602-0001	Broome	River	1.0	Mi.	D	Fish Survival	Precluded	Metals
CANASAWACTA CRBBK	0602-0013	Chenango	River	7.0	Mi.	В	Pish Propagation	Stressed	Silt (Sediment)
CHBNANGO RIVER	0602-0009	Chenango	River	45.0	Mi.	B,C,BT	Fish Propagation	Stressed	Nutrients
CHBNANGO RIVER	0602-0033	Broome	River	10.0	Mi.	В	Fish Propagation	Stressed	Nutrients
CHBNANGO RIVER	0602-0050	Broome	River	1.0	Mi.	B	Pish Propagation	Stressed	Metals
COLD BROOK	0602-0011	Chenango	River	3.0	Mi.	C(T)	Aesthetics	Stressed	Silt (Sediment)
DUDLEY CREEK	0602-0037	Broome	River	5.0	Mi.	C,C(T)	Pish Propagation	Stressed	silt (Sediment)
B.BR.TIOUGHNIOGA	0602-0020	Cortland	River	20.0	Mi.	C(T)	Boating	Stressed	Silt (Sediment)
BATON BROOK RES.	0602-0041	Hadison	Lake	35.0	A	B	Bathing	Threatened	Nutrients
PABIUS BROOK	0602-0026	Onondaga	River	3.0	Mi.	C(T)	Fish Propagation	Threatened	Thermal Changes
FACTORY BROOK	0602-0025	Cortland	River	3.5	Mi.	C(TS)	Fish Survival	Threatened	Silt (Sediment)
PLY CREEK	0602-0012	Chenango	River	2.0	Mi.	C(T)	Aesthetics	Threatened	Nutriento
GORTON LAKE	0602-0040	Madison	Lake	7.0	A	В	Boating	Stressed	Nutrients
HUNT CREEK	0602-0051	Madison	River	0.5	Mi.	C(T)	Aesthetics	Threatened	Nutrients
LAKE MORAINE	0602-0007	Madison	Lake	235.0	A	B	Boating	Impaired	Nutrients
NORWICH RESERVOIR	0602-0010	Chenango	Lake (R)	15.0	A	A	Water Supply	Stressed	Nutrients
OSBORNE CREEK	0602-0030	Broome	River	3.0	Mi.	c	Pish Propagation	Stressed	Silt (Sediment)
OTSELIC RIVER	0602-0015	Chenango	River	15.0	Mi.	C(T)	Fish Propagation	Strenged	Thermal Changes
OTSELIC RIVER	0602-0024	Cortland	River	14.5	Mi.	C(T)	Pich Survival	Stressed	Silt (Sediment)
OTSELIC RIVER	0602-0028	Broome	River	1.0	Mi.	c	Pich Survival	Strepped	silt (Sediment)
OTSELIC RIVER	0602-0043	Madison	River	6.0	Mi.	cr, c	Fish Propagation	Strepped	Thermal Changes
PAGE BROOK	0602-0029	Broome	River	5.0	Hi.	c	Pish Propagation	Impaired	Nutriento
PAGE BROOK	0602-0036	Broome	River	5.0	Mi.	c	Aesthetics	Strenned	silt (Sediment)
PAYNE BROOK	0602-0003	Madison	River	2.2	Mi.	B(T)	Pishing	Precluded	Oxygen Demand
PHELPS CREEK	0602-0035	Broome	River	3.0	Mi.	c	Aesthetics	Stressed	Silt (Sediment)
PLYMOUTH RESERVR.	0602-0014	Chenango	Lake	78.0	A	В	Boating	Strepped	Nutriento
SONG LAKE	0602-0019	Cortland	Lake	109.0	A	В	Bathing	Impaired	Nutrients
TIOUGHNIOGA RIVER	0602-0002	Cortland	River	13.5	Mi.	B(T)	Piohing	Impaired	Other Pollutant
TULLY LAKE	0602-0018	Cortland	Lake '	115.0	A	В	Bathing	Impaired	Nutrients

(continued on next page...)

Reference 33



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FINAL

Remedial Investigation Report Rosen Site Cortland, New York

Volume 1 of 3

Contributing Potentially Responsible Parties

October 1992 (Revised May 1994)

BLASLAND, BOUCK & LEE, INC. ENGINEERS & SCIENTISTS

> 6723 Towpeth Road Syracuse, New York 13214 (315) 446-9120

APPENDIX B

Regional Well Evaluation

The purposes of this appendix are to provide:

- A summary of regional wells in the Cortland area, including locations and specifications;
- A summary of regional water quality data for volatile organic compounds (VOCs);
- An interpretation of the regional water quality in relation to the water quality observed at the Rosen Site.

While the first two purposes of this appendix are straightforward, the latter purpose required careful evaluation of information beyond just the water quality data including well construction and location data, historical and current industries and entities in the Cortland area, regional hydrogeologic characteristics, and/or general hydrogeologic principles.

Investigations, reports, and/or data referenced in this appendix are fully documented in Section 5 of this Remedial Investigation Report.

Regional Well Locations and Specifications

There are approximately 150 ground-water wells (excluding those installed during the Remedial Investigation [RI] at the Rosen Site) in the Cortland area. These wells serve a variety of functions. Most wells are ground water observation points, and a few serve as industrial and domestic water supplies. The ground-water observation or monitoring wells are, for the most part, associated with the Otter Creek - Dry Creek Remedial Investigation of potential ground-water impacts reportedly associated with the Smith Corona plant, located in South Cortland. Other monitoring wells present in the Cortland area were installed as a part of underground storage tank investigations, property audits, United States Geological Survey (USGS) hydrogeologic studies, and/or other investigations.

Of these 150 wells, approximately a third, or about 45 wells, are located in areas potentially hydraulically downgradient of the Rosen Site. In addition, approximately 8 wells are located in the proximity of the Rosen Site in areas believed to be hydraulically upgradient of the site. The well construction specifications, if known and available, of these 53 wells are summarized in Table B-1. The approximate locations of these 53 wells are depicted on Figure B-1.

The majority of these 53 wells were installed under the oversight of the USGS or the Cortland County Planning Department (CCPD). Those wells not installed under the USGS or CCPD are discussed below:

- S&B Roofing Wells #1 through #3 were installed in March 1989 by Empire Soils Investigations, Inc. (Empire) as a part of an environmental site assessment for Wallace Industries, Inc (Empire, May 1989). These wells monitor the upper outwash unit.
- Cortland County Recycling Center Wells MW-1 and MW-2 were installed on September 8, 1989 by North Star Drilling Co. for Barton & Loguidice, P.C. (North Star, September 1989). These wells are installed in the upper outwash unit.

- Christian Assembly of God Church Wells 1 and 2 as well as Test Pit 2 were installed in 1990 by Buck Engineering as a part of a site audit for the Church. These wells were installed in test pits, and no construction specifications were provided (Buck Engineering, March 1990 and June 1990). These wells are believed to monitor the upper outwash unit.
- Public Works Garage Wells MW-1 through MW-3 were installed by Buck Engineering as a part of a "Subsurface Contaminant Investigation" for the City of Cortland Department of Public Works. These wells were installed in test pits, and no construction specifications were provided (Buck Engineering, October 1990). These wells are believed to be installed in the upper outwash unit.
- Public Works Garage Well MW-7 and Fire Station Wells MW-8 and MW-9 were installed under the oversight of Resource Associates for the City of Cortland as a part of a underground storage tank (UST) investigation associated with New York State Department of Environmental Conservation (NYSDEC) Spills No. 91-08345 and 91-08299 (Resource Associates, February 1992). These wells are installed in the upper outwash unit.
- Hess Wells MW-1 through MW-9 were monitored and/or installed under the direction of Stearns & Wheler for the Amerada Hess Corporation as a part of a ground-water investigation at former Hess Station 32252 (Stearns & Wheler, September 1991 and January 1992). These wells are installed in the upper outwash unit.
- Dowser Wells #2A and #3 were installed in January 1992 by North Star Drilling under the direction of Buck Engineering for Dowser Electric as a part an environmental investigation (Buck Engineering, April 1992). The installation dates of and well specifications for Dowser Wells #1 and #2 are not known.
- Wells #173 and #176 are believed to be former industrial water supply wells at the former location of the Brewer Titchener Corporation and the present location of the Rubbermaid Corporation. Both these wells are located within the lower outwash unit.
- Wells #181 and the ETL wells are believed to be current industrial water supply wells at the present locations of the Brewer Titchener Corporation and ETL Testing Laboratories, Inc. Both these wells are located within the upper outwash unit.

Regional Water Quality

There have been several regional ground-water sampling and analysis events conducted in the Cortland area. The analytical results obtained from these events as reported are discussed below. It should be noted that these results were not reported with detailed quality assurance/quality control (QA/QC) data. Therefore, these analytical data have not been validated as a part of this evaluation nor were these analytical data believed to be validated as a part of the regional sampling and analytical events discussed.

The majority of the 53 wells described in the section above have been sampled as part of the Otter Creek - Dry Creek Remedial Investigation. The CCPD and the USGS have conducted two sampling and analytical events, one in April 1990 and one in September 1990 for this investigation. In April 1990, ground-water samples were collected from wells within the Cortland area by representatives of the USGS. These samples were analyzed by Dr. Peter M. Jeffers of SUNY College at Cortland, Department of Chemistry. Ground-water samples were collected and analyzed by Dr. Peter M. Jeffers from Cortland area wells again in September 1990.

Samples were analyzed for dichloroethene; 1,1,1-trichloroethane; trichloroethene; tetrachloroethene;

1,1-, cis-, and trans- isomers of dichloroethene; benzene; toluene; xylene; 1,4-dichlorobenzene; bromodichloromethane; methylene chloride; trichloromethane; and carbon tetrachloride.

Dr. Jeffers used four different column/detector combinations in the analysis of these ground-water samples:

- A 20-inch UCW-98 column with a Hall electrolytic conductivity detector;
- A 6-foot Poracil-C/n-octane column with a Hall electrolytic conductivity detector for the laboratory and field spike comparisons;
- The UCW-98 followed by the Porcil column, with a Hall electrolytic conductivity detector.
- A final set of analyses utilized the combined column pair with a hydrogen flame ionization detector.

The results of the April 1990 and the September 1990 CCPD/USGS sampling events (Jeffers, May 1990 and Jeffers, September 1990) are provided in Table B-1 for the 45 wells potentially hydraulically downgradient of the Rosen Site and the 8 wells believed to be hydraulically upgradient and near the Rosen Site. Table B-2 sets forth the results of all wells monitored as part of the Otter Creek - Dry Creek Remedial Investigation.

Of the chlorinated and aromatic hydrocarbons analyzed, predominantly trichloroethene and 1,1,1 trichloroethane were detected. The highest concentrations of trichloroethene were detected at the monitoring wells near the Smith Corona plant. Of the wells believed to be hydraulically downgradient of the Rosen Site, relatively higher concentrations of trichloroethene (15.2 ug/L) and 1,1,1-trichloroethane (107.5 ug/L) were detected at USGS 90-1S, which is located at the city of Cortland Public Works Garage. Dichloroethanes and dichloroethenes were also detected at this well at concentrations of 25 ug/L and 28.9 ug/L, respectively. Concentrations ranging from 15 ug/L to 1 ug/L of trichloroethene, 1,1,1-trichloroethane, and other chlorinated hydrocarbons were detected at USGS 90-2 (#335), 2821 Kellog (#346). Concentrations generally near or less than 1 ug/L of trichloroethene, 1,1,1-trichloroethane, and/or other chlorinated hydrocarbons were detected at 53 Greenbush (#366), Old Hess (MW-3), the Cortland County Office Building well (#177), and Elm B (#282). Trichloroethene, 1,1,1-trichloroethane, and other chlorinated hydrocarbons were also detected at the hydraulically upgradient wells Dowser #1, #2, #2A, and #3 at concentrations ranging from less than 1 ug/L to 18.7 ug/L.

Other sampling and analysis events not conducted by the CCPD and/or the USGS in the Cortland area are also summarized in Table B-1 and described below:

- S&B Roofing Wells #1 through #3 were sampled on March 21, 1989 by Empire and analyzed by Huntington Analytical Services for purgeable halocarbons by EPA Method 601, purgeable aromatics by EPA Method 602, organochlorine pesticides and PCBs by EPA Method 608, and semivolatile priority pollutants by EPA Method 625 (Empire, May 1989). 1,1,1-trichloroethane and 1,1-dichloroethane were the predominant constituents detected. Concentrations ranged from 360 ug/L to 430 ug/L of 1,1,1-trichloroethane and 75 ug/L to 100 ug/L of 1,1-dichloroethane. In addition, several dichloroethenes, trichloroethene, tetrachloroethenes, and 1,2-dichloroethane were detected.
- Recycling Center wells #1 and #2 were sampled by Buck Engineering on September 8, 1989 and analyzed by Buck Environmental Laboratories, Inc. for purgeable halocarbons by EPA Method 601 (Buck Environmental Laboratories, Inc. 1989). Both 1,1,1-trichloroethane (23.2 ug/L and 43.2 ug/L) and 1,1-dichloroethane (ND to 2.5 ug/L) were detected.

- Christian Assembly of God Church Wells 1 and 2 were sampled on June 6, 1990 and ground water from Test Pits 1 and 2 was sampled on March 18, 1990 by Buck Engineering (Buck Engineering, March 1990 and June 1990). These samples were analyzed for benzene, toluene, and xylenes (BTX) by EPA Method 602 and purgeable halocarbons by EPA Method 601. 1,1,1-trichloroethane (29 ug/L and 49.5 ug/L), 1,1-dichloroethane (4.7 ug/L and 10.7 ug/L), trichloroethene (7.0 ug/L and 7.4 ug/L), and tetrachloroethene (1.2 ug/L and 1.0 ug/L) were detected at Test Pit 1 and Well 1. No constituents were detected at Test Pit 2 and Well 2.
- Public Works Garage Wells MW-1 through MW-3 and USGS 90-1-S (MW-4) were sampled by Buck Engineering on September 18, 1990 and October 3, 1990 and analyzed by Buck Environmental Laboratories, Inc. for volatile aromatic and unsaturated organics by EPA Method 503.1 and purgeable halocarbons by EPA Method 601 (Buck Engineering, October 1990). 1,1,1-trichloroethane (30.2 ug/L to 331.0 ug/L), 1,1-dichloroethane (3.9 ug/L to 51.6 ug/L), and trichloroethene (3.0 ug/L to 15.9 ug/L) were detected at these wells.
- Public Works Garage Wells MW-1, MW-3, MW-4 (USGS-90-1S), and MW-7, and Fire Station Wells MW-8 and MW-9 were sampled by Buck Engineering on December 11, 1991 and analyzed by Buck Environmental Laboratories, Inc. for volatile aromatic and unsaturated organics by EPA Method 503.1 (Resource Associates, February 1992). Trichloroethene was detected in concentrations ranging from 3.3 ug/L to 14.1 ug/L.
- Hess Wells MW-1 through MW-9 were sampled by Stearns & Wheler and analyzed by Toxikon for purgeable aromatics by EPA Method 602 (Stearns & Wheler, September 1991 and January 1992). Benzene, toluene, ethylbenzene, and xylenes (BTEX) were detected.
- Dowser Wells #1, #2A, and #3 were sampled by Buck Engineering on February 24, 1992 and analyzed by Buck Environmental Laboratories, Inc. for VOCs by EPA Method 502.2 (Buck Engineering, April 1992). 1,1,1-trichloroethane was detected at two of these wells at concentrations of 6.6 ug/L and 18.7 ug/L.

Interpretation of Regional Water Quality in Relation to the Rosen Site

Prior to interpreting the regional water quality data in relation to the Rosen Site, this section first summarizes the relevant hydrogeologic and water quality data obtained during the Remedial Investigation (RI) at the Rosen Site:

- 1,1,1-trichloroethane and its degradation product, 1,1-dichloroethane are the predominant constituents detected at the Rosen Site. These VOCs were detected within the upper outwash ground-water flow system associated with the site. Concentrations of 1,1,1-trichloroethane ranged from 3,400 ug/L to 1,100 ug/L at the interior of the site and 4 ug/L to 270 ug/L at the downgradient perimeter of the site. Concentrations of 1,1-dichloroethane ranged from 430 ug/L to 340 ug/L at the interior and 2 ug/L to 100 ug/L at the downgradient perimeter of the site.
- The distribution of these constituents indicates that 1,1,1-trichloroethane and 1,1-dichloroethane have not migrated into the confining unit nor the lower outwash unit. Further, the highest concentrations of these constituents are detected in the lower portion of the upper outwash unit, above the confining unit, at both the northern perimeter of the site and off-site along Huntington Street. The saturated thickness of the upper outwash unit ranges from approximately 35 to 55 feet at these locations.
- Ground-water flow in the upper outwash is toward the northeast both at the site and off-site along Huntington Street.

Therefore, in consideration of the above RI results, regional wells, if any, that would be affected by the Rosen Site would be expected to have concentrations of 1,1,1-trichloroethane and 1,1-dichloroethane less than the concentrations detected at the site (due to dispersion and dilution), to have low to no concentrations of these constituents in the upper portion of the upper outwash unit (due to dilution from recharging precipitation), and to be located to the northeast of the site, the hydraulically downgradient direction.

As observed during the RI, the highest concentrations of 1,1,1-trichloroethane and 1,1-dichloroethane were detected near potential source areas of these constituents at the site. At the perimeter of the site and off-site along Huntington Street, the concentrations of 1,1,1-trichloroethane and 1,1-dichloroethane decrease by a half order of magnitude, and the highest concentrations are observed within the lowermost 10 to 20 feet of the upper outwash unit as opposed to within the uppermost 10 feet of the upper outwash unit.

Further from the site, directly recharging waters from precipitation would be expected to further direct the ground water containing 1,1,1-trichloroethane and 1,1-dichloroethane vertically downward in the upper outwash, additional lateral dispersion would occur as a result of ground-water migration, and additional mixing with other ground waters originating from other recharge areas within the upper outwash would occur causing further dilution of these constituents in the ground water. This expectation of a decrease in concentrations further from the potential source areas is based on observations at the site and general hydrogeologic principles. However, any interpretations will be tempered by the recognition that the distribution of 1,1,1-trichloroethane and 1,1-dichloroethane associated with the site has only been investigated in detail recently during the RI and may have been different in the past. In addition, there would be a time lag between the distribution of constituents at the site and their potential distribution off-site as a result of migration within the ground-water flow system.

Although the direction of ground-water flow observed at the Rosen Site is toward the northeast, further north of the site and along Huntington Street, the direction of ground-water flow likely becomes more easterly. Current updated regional ground-water flow studies and models are in progress by the USGS. Past USGS models have depicted ground-water flow at the Rosen Site as more easterly than northerly. Ground-water elevations obtained during the RI at the Rosen Site clearly show ground-water flow to the northeast. The updated USGS model will utilize the ground-water elevations obtained during the Rosen Site RI.

In light of the above, an interpretation of the regional water quality data collected by the CCPD, USGS, and others from the 53 regional wells in the proximity of the Rosen Site is presented below.

The S&B Roofing Wells #1 through #3 are the closest regional wells to the site and are located to the northeast just across Pendleton Street. These wells are believed to be hydraulically downgradient of Rosen Site. These wells are installed in the uppermost 5 feet of the saturated upper outwash (Note: The screens span a depth of 5 to 20 feet below ground level, and the top of the water column was observed to be about 14 to 15 feet below ground level). Both 1,1,1 trichloroethane (360 to 410 ug/L) and 1,1-dichloroethane (75 to 100 ug/L) were detected at essentially the same concentration at each location. These constituents could have plausibly originated at the Rosen Site.

It should be noted that observations made during the Environmental Site Assessment for the S&B Roofing property, suggest that there may be sources of VOCs, including 1,1,1-trichloroethane and 1,1-dichloroethane, and/or other constituents at the S&B Roofing property as follows:

- Four to 5 feet of fill (described as predominantly construction debris) is present throughout much of the S&B property.
- A "kerosene" odor was noted during the installation of test pit TP-3 in the middle of the property.

- 1,1,1-trichloroethane was detected in a soil sample from test pit TP-3 (95 ug/kg).
- Photoionization detector (PID) measurements were observed above 1 ppm in the unsaturated soils at the boring for MW-3 (Empire, May 1989).
- Cortland County Recycling Center wells #1 and #2 do not appear to be hydraulically downgradient of the Rosen Site, i.e., they are more east of the site than the northeast. However, these wells may be hydraulically downgradient as the northeast ground-water flow component observed at the Rosen Site becomes more easterly further north of the site. These wells are about 1,000 feet from the site and are screened in the upper 10 feet of the upper outwash. 1,1,1 trichloroethane (23.2 ug/L and 43.2 ug/L) and 1,1-dichloroethane (2.5 ug/L) were detected at these wells. These constituents could have plausibly originated at the Rosen Site.

It should be noted that there were observations made during the installation of these wells (North Star, September 1989), which suggest that there may be sources of VOCs and/or other constituents at the Cortland County Recycling Center as follows:

- About 3 feet of fill (described as coal, ash, and cinders) is present at the property.
- The recycling center is adjacent to a former sheet metal company, a former metal recovery operation, and a current auto finisher. These types of operations commonly used 1,1,1-trichloroethane as a degreasing/cleaning solvent.
- The Christian Assembly of God Wells 1 and 2 are approximately 1,800 feet northeast and hydraulically downgradient of the Rosen Site. These wells are installed in test pits, assumed to be within the upper few feet of the upper outwash. At this distance from the site, the concentrations of 1,1,1-trichloroethane and 1,1-dichloroethane in the uppermost section of the water column would be expected to be non-detect or lower than those detected (1,1,1-trichloroethane: 29 ug/L and 49.5 ug/L; 1,1-dichloroethane: 4.7 ug/L and 10.7 ug/L) in the ground water from Test Pit 1 and Well 1.

It should be noted that observations made during the site audit suggest that there may be sources of VOCs and/or other constituents in this area as follows:

- Up to 7 feet of fill (described as black soil, asphalt, concrete, metal fragments, bricks, ash, and small debris) is present at the property.
- A portion of the property was used as a salvage yard, and a portion formerly housed a factory building that burned down.
- The site audit hypothesizes that the source of the VOCs detected could be within the fill at the property or could originate from other past and present industries/entities including the Rosen Site and the City of Cortland Public Works Garage.
- The Public Work Garage Wells MW-1, MW-2 and MW-3 are approximately 2,000 feet northeast of the Rosen Site and are believed to be hydraulically downgradient. These wells are installed in test pits, assumed to be within the upper few feet of the upper outwash. At this distance from the site, the concentrations of 1,1,1-trichloroethane and 1,1-dichloroethane in the uppermost section of the water column at these wells would be expected to be non-detect or at lower concentrations than those detected at wells MW-1 and MW-3. USGS-90-1S (MW-4) was sampled at the

same time as wells MW-1 through MW-3. This well is installed deeper in the upper outwash and screened from 30 to 34 feet below ground level. The bottom of the upper outwash in this area is about 64 feet below ground level, where the silt-clay confining unit is present. The 1,1,1-trichloroethane (86.5 ug/L to 174 ug/L) and 1,1-dichloroethane (25.0 ug/L to 32.1 ug/L) at the concentrations detected at USGS-90-1S could have plausibly originated from the Rosen Site. However, the higher concentrations of these constituents detected at MW-3 (331 ug/L for 1,1,1-trichloroethane and 51.6 ug/L for 1,1-dichloroethane) and comparable concentrations at MW-1 (73.8 ug/L for 1,1,1-trichloroethane and 12.4 ug/L for 1,1-dichloroethane), both monitoring the upper few feet of the upper outwash or about 15 to 20 feet higher than USGS-90-1S, suggest that there may be sources of these constituents at or in the area of the Public Works Garage.

The "Subsurface Contaminant Investigation" Report (Buck Engineering, October 1990) for the Public Works Garage hypothesizes that the contamination may originate, in part, from the former channel of Perplexity Creek. Reportedly, Perplexity Creek was rerouted in 1939, well before Rosen operations. Therefore, this hypothesized surface water pathway is unlikely since Perplexity Creek currently flows directly east of the Rosen Site and not to the northeast.

- USGS 90-1D and 91-1 are also installed at the Public Works Garage. These wells monitor the silt-clay confining unit and the bedrock, respectively. Reportedly, no VOCs were detected at USGS 91-1, and USGS 90-1D has not been sampled. These wells would not be expected to be affected by the Rosen Site, as they monitor the confining unit that has not been affected at the site and the bedrock unit below the confining unit.
- Public Works Wells MW-7, MW-8, and MW-9 are hydraulically downgradient of the Rosen Site. However, each of these wells is installed within the uppermost 10 to 15 feet of the upper outwash, which may be too shallow to monitor ground waters potentially affected by the Rosen Site. The ground water from these wells has not been analyzed by a method which quantifies 1,1,1-trichloroethane and 1,1-dichloroethane.
- The well at 53 Greenbush does not appear to be hydraulically downgradient of the Rosen Site. Although low concentrations (<1 ug/L) of VOCs have been detected at this well, the concentrations of these VOCs may be due to regional background water quality.
- Wells #173 and #176 are believed to be installed within the lower outwash. These wells would not be expected to be affected by the Rosen Site, as they monitor the lower outwash unit beneath the confining unit. Both the confining unit and the lower outwash have not been affected at the Rosen site. Wells #173 and #176 have not been sampled.
- The Hess wells are northeast of the Rosen Site; however, they are likely not hydraulically downgradient of the site. Only low concentrations (< 1 ug/L) of VOCs have been detected at Hess well MW-3. Although ground water from the other Hess wells has not been analyzed for chlorinated VOCs, the concentrations would be expected to be similar to those detected at MW-3. The concentrations of these VOCs may be due to regional background water quality.
- USGS 91-4S and #177 located at the Cortland County Office Building do not appear to be hydraulically downgradient of the Rosen Site. Although low concentrations (<1 ug/L) of VOCs have been detected at these wells, the concentrations of these VOCs may be due to regional background water quality.

- USGS 90-2, Elm A and B, #181, and Yaman A and B do not appear to be hydraulically downgradient of the Rosen Site. Although low concentrations (<1 ug/L) of VOCs have been detected at two of these wells, the concentrations of these VOCs may be due to regional background water quality.
- The 2821 Kellog well (#346) does not appear to hydraulically downgradient of the Rosen Site. However, this well may be hydraulically downgradient as the northeast ground-water flow component observed at the site becomes more easterly and southeasterly near the Tioughnioga River. This well is installed in the upper outwash and screened from 20 to 24 feet below ground level. The bottom of the upper outwash in this area is about 43 feet below ground level, where the silt-clay confining unit is encountered. The 1,1,1-trichloroethane and 1,1-dichloroethane at the concentrations detected at 2821 Kellog (11.5 ug/L to 13.8 ug/L for 1,1,1-trichloroethane and 5.2 ug/L for 1,1-dichloroethane) could be due to regional background water quality based on the concentrations of these VOCs detected at wells hydraulically upgradient of the Rosen Site.
- The ETL wells do not appear to be hydraulically downgradient of the Rosen Site. These wells are east of the Tioughnioga River and would be likely not be impacted since most ground waters discharge to the Tioughnioga River in this area.
- Wells Dowser #1 through #3 and CT-3S, CT-3D, and CT-11 are located hydraulically upgradient of the Rosen Site. These wells are believed to be installed within the upper outwash. VOCs including 1,1,1-trichloroethane and 1,1-dichloroethane have been detected at these wells at concentrations ranging from 0.5 ug/L to 18.7 ug/L and 0.4 ug/L to 0.5 ug/L, respectively. These concentrations suggest that there are upgradient sources of these VOCs, and the concentrations of these VOCs may be due to background regional water quality.

Hydraulically	. X-						Analytical Results		Analytical
Downgradient Wells	Approximate Distance (fi) and Direction from Rosen Site	Geology	Formation Screened	Screen Interval (Feet BGL)	Suitability of Well Use	Date	Constituents Detected	Concentration (ug/L)	Method
S&B Rooting #1	100 NE	0-2 ft. Fill	Upper Outwash	5-20	May be suitable upon	3/89	1,1,1-trichloroethane	360	EPA 601, 602
San Housing .		2-20 ft. Silt, fine to coarse sand grades to sand and gravel			inspection.		t.1-dichloroethene	7 5	
							1,1-dichloroethane	75	
S&B Roofing #2	500 NE	0-0.5 ft. Topsoil	Upper Outwash	5-20	May be suitable upon	3/89	trichioroethene	2.7	EPA 601, 602, 608
San Hooming #2		2-20 ft. Silt, fine to coarse sand grades to sand and gravel.			inspection.		1,1,1-trichloroethane	430	
							1,1-dichloroethane	85	
							1,1-dichloroethene	11	
S&B Rooling #3	650 NE	0-2 ft. Sand and silt	Upper Outwash	5-20	May be suitable upon	3/89	trichhloroethene	19	EPA 601, 602, 608
SAB Hooling #3		2-20 ft. Sand and gravel			inspection.		1,1,1-trichloroethane	410	
					,		tetrachloroethene	1.1	
							1.1-dichloroethane	100	
							1,1-dichtoroethene	13	
			·				1,2-dichtoroethane	ı	
							trans 1,2-dichlorethene	2	
		•					1,1,2,2-tetrachloroethene	1.1	
Recycling Center	1,000 E-NE	0-22 ft. Sand & Gravel	Upper Outwash	12-22	May be suitable upon inspection.	9/08/89	1,1,1-trichloroethane		EPA 601
Well #1 Recycling Center	1,000 E-NE	0-26 ft. Sand & Gravel	Upper Outwash	18-26	May be suitable upon inspection.	9/08/89	1,1-dichloroethane	2 5	EPA 601
Well #2					inspection.		1,1,1-trichforoethane	43 2	

Hydraulically							Analytical Results		Analytical
Downgradient Wells	Approximate Distance (ft) and Direction from Rosen Site	Geology	Formation Screened	Screen Interval (Feet BGL)	Sultability of Well Use	Date	Constituents Detected	Concentration (ug/L)	Method
Christian Assembly	1,800 NE	0-3 ft. Soil and Cobbles	Upper Outwash(?)	No Information	Not suitable due to	3/18/90	1,1,1-trichloroethane	29 0	EPA 601/602
of God Church Well 1		3-7 ft. Fill (ashes, wood) 7-8 ft. Till (?)			installation methods. Shallow well installed in		trichloroethene	7.0	
					test plt.		1,1-dichloroethane	4.7	
							tetrachloroethene	1.2	
ĺ						6/6/90	1,1-dichloroethane	10.7	EPA 601/602
							1,1,1-trichloroethane	49 5	!
							trichloroethene	7.4	
							tetrachtoroethene	10	
Christian Assembly of God Church Well 2	1,800 NE	No well log	No Information		Not sultable due to installation methods. Shallow well installation in test pit.	6/6/90	ND		EPA 601/602
Christian Assembly of God Church Test Pit 2		0-3 ft. Soil and cobbles 3-9 ft. Fill (ash, asphalt, concrete and soil) 9-10 ft. Till (?)	•••		Test pit	3/18/90	ND		EPA 601/602
Public Works Garage	2,000 NE	Probably Sand & Gravel based on USGS 90-15	No information		Not suitable due to installation methods.	9/18/90	1,1-dichloroethane		EPA 601
MW-1		0000 00-10			Shallow well installed using an excavator.		1,1,1-trichloroethane	73.8	
					an excession.		trichloroethene	9.0	
				•		9/18/90	trichloroethene	8.5	EPA 503 1
						12/11/91	trichloroethene	13 6	EPA 503 1
Public Works Garage			No information		Not suitable due lo	10/3/90	1,1-dichloroethane	3 9	EPA 601
MW-2		USGS 90-1S			installation mathods. Shallow well installed using		1,1,1-trichloroethane	30 2	
					an excavator.		trichioroethene	3.0	

Hydraulically							Analytical Results		Analytical
Downgradient Wells	Approximate Distance (ft) and Direction from Rosen Site	Geology	Formation Screened	Screen Interval (Feet BGL)	Sultability of Well Use	Date	Constituents Detected	Concentration (ug/L)	Method
Public Works Garage	2,000 NE		No information	No information	Not suitable due to	9/18/90	1,1-dichloroethane	51.6	EPA 601
MW-3		USGS 90-15			instaliation methods. Shallow well. Installed using an excavator.		1,1,1-trichloroethane	3310	
					deling an excertation.		trichloroethene	8 2	
						9/18/90	trichloroethene	8.8	EPA 503 I
							trichloroethene	L	EPA 503.1
(#334) (Public		0-64 ft. Sand & Gravel 64-107 ft. Silt & Clay	Confining Unit	93-98	May be suitable upon inspection.	NS	NS	NS	หร
Works Garage MW-5) USGS 90-15			Upper Outwash		May be suitable upon	9/18/90	1,1-dichloroethane	36 1	EPA 601
(#332) (Public Works					Inspection.		1,1,1-trichloroethane	86 5	
Garage MW-4)							trichloroethene	13 0	
							methylene chloride trichloroethene	'trace' 14.0	EPA 503 1
							1,1,1-trichloroethane	107.5	Jeffers
							trans 1,2-dichloroethene	28 9	
							trichloroethene	15.2	
							1,1-dichloroethane	25.0	
							loluene	2.0	EPA 601
j						10/1/90	1,1-dichloroethane	37 1	
	:						1,1,1-trichloroethane	174	
							trichloroethene	15.9	
						12/10/91	trichloroethene	13.8	EPA 503 1

Hydraulically	`						Analytical Results		Analytical
Downgradient Wells	Approximate Distance (ft) and Direction from Rosen Site	Geology	Formation Screened	Screen Interval (Feet BGL)	Sultability of Well Use	Date	Constituents Detected	Concentration (ug/L)	Method
USGS 91-1 (Public Works Garage MW-6)		0-64 ft. Send & Gravel 64-138 ft. Silt/Clay 138-149 ft. Gravel 149-180 ft. Fine to medium Sand, some silt 180-184 ft. Fine gravel and coarse Send 184-227 ft. Gravel 227-234 ft. Silt/Send 234-280 ft. Pebbly fine to medium Sand, some silt 280-313 ft. Cobble gravel 313 ft. Olive gray Siltstone/Sandstone	Bedrock 	Open Ended at 310		1/16/91	According to USEPA - ND		Jeffers
Public Works Garage (MW-7)	2,000 NE	0-25 ft. Gravel	Upper Outwash	10-25	Well may be suitable upon inspection.	12/11/91	trichloroethene	14.1	EPA 503 1
	2,500 NE	0-14 ft. Sand & Gravel	Upper Outwash		Well may be suitable upon inspection.	12/11/91	trichloroethene	3.3	EPA 503 1
Firestation (MW-9)	2,500 NE	0-20 ft. Sand and Gravel	Upper Outwash		Well may be suitable upon inspection.	12/11/91	trichloroethene	4 4	EPA 503 1
53 Greenbush (#366)	2,000 N	No information	No information		May be suitable upon inspection; however, no Information on construction	Sample 2	1,1,1-trichloroethane trichloroethene	0 2 0 2 0 7 0 5	Jeffert
						Sample 2	tetrachloroethene	0 75 0 4	
							cis 1,2-dichtoroethene 1,1,1-trichtoroethane	0 1 0 4	
							trichloroethene	0.8	
,							tetrachforoethene	0.5	<u> </u>
#173	2,250 NE	Depth 155 ft.; ends in Sand.	No information		Probable former production well at Brewer Titchener.	NS	NS	NS	NS . 0

	e de maria de la compansión de la compan	t v					Analytical Results		Analytical
Hydraulically Downgradient Wells	Approximate Distance (ft) and Direction from Rosen Site	Geology	Formation Screened	Screen Interval (Feet BGL)	Suitability of Well Use	Date	Constituents Detected	Concentration (ug/L)	Method
C-17		0-35 ft. Gravel 35-96 ft. Silt 96-103 ft. Hard plan or rock.	No Information	No information		NS	NS		ns
Old Hess (MW-3) (#365)	2,500 NE		No Information	No information	May be suitable upon inspection; however, no information on construction.	9/90 4/90 9/90 4/90 9/90	1,1-dichloroethene 1,1,1-trichloroethane toluene xylene benzene trans 1,2-dichloroethene	0.3 0.5 0.3 4.5 42.5 2.0 5.3 **trace** 2.1	Jellers
						•	cls 1,2-dichloroethene trichloroethene	0.06 0.1	

Hydraulically	ł			ļ			Analytical Results	·	Analytical
Downgradient Wells	Approximate Distance (ft) and Direction from Rosen Site	Geology	Formation Screened	Screen Interval (Feet BGL)	Suitability of Well Use	Date	Constituents Detected	Concentration (ug/L)	Method
Hess Wells MW-1,	2,500 NE	0-15 ft. Sand, some Gravel		All approximately			benzene	27.1 to 689	EPA 602
MW-2, MW-3, MW-4, MW-5, MW-6				5 to 14.5		(MW-1 through MW-4 only)	toluene	20.5 to 4.610	
						MINI-4 OIIIY)	ethylbenzene	50 7 to 1,740	
			No.				xylenes	664.3 to 11,720 ND-250	EPA 602
							benzene	ND-2,700	
i							loluene	ND-250	
							ethylbenzene xylenes	ND-7,900	
			<u>.</u>			11/91	benzene	ND 1,180	
							toluene	ND-6,360	
]			ethylbenzene	ND-2,700	
							xylenes	N() 11,700	
Hess Wells MW-7,	2,500 NE	0-17 ft. Sand & Gravel	Upper Outwash	6 to 16 or 7 to			benzene	4 to 45	EPA 602
MW-8 and MW-9				17			toluene	4 lo 23	
							ethylbenzene	3 10 38	
							xylenes	45 to 87	
Rubbermaid (#176)		0-35 ft. Sand & Gravel 35-130 ft. Clay 130-185 ft. Gravel	No information	No information	Probable Former Production Well	NS	NS	NS	NS
Cortland County Office Building USGS 91-4(S)	3,500 N		Upper Outwash	23-33	May be suitable upon inspection.		NS		

Hydraulically							Analytical Results		Anglytical
Downgradient Wells	Approximate Distance (ft) and Direction from Rosen Site	Geology	Formation Screened	Screen Interval (Feet BGL)	Suitability of Well Use	Date	Constituents Detected	Concentration (ug/L)	Method
Cortland County Office Building		5-16 ft. Clay & Gravel	No Information		May be suitable upon inspection.	4/90	'trace' toluene	0.3	Jeffers
(#177)		16-55 ft. Sand & Gravel 55-85 ft. Clay 85-102 ft. Sand & Gravel				l	letrachloroethene	0.25	
							trichloromethane	0.7	
							carbon tetrachloride	<0.2	
USGS 90-2 (#335)	3.750 NE	0-46 ft. Sand & Gravel	Upper Outwash	30-34	May be suitable upon	9/90	1,1,1-trichloroethane	2.7	Jeffers
0202 80.5 (#333)		46-50 ft. Fine Sand and Silt 50-56 ft. Silt and Clay 56-68.5 ft. Till, Bedrock @ 65.5 feet			inspection.		trichloroethene	0.1	
2821 Kellogg (#346)	4,250 E	0-37.5 ft. Sand & Gravet 37.5-43 ft. Clay, Silt, and Fine Sand	Upper Outwash	20-24	May be suitable upon inspection.	4/90 Sample 1 Sample 2	1,1-dichloroethene	1 1 3.0	Jeffers
		43-99 ft. Clay, Silty Clay	<i>.</i>			Sample 2	1,1,1-trichloroethane	11.5 15.0	
							'Trace' toluene	1.4	
						9/90	1,1,1-trichloroethane	13.8	
							trans 1,2-dichloroethene	0.6	
							1,1-dichloroethane	5.2	
Elm A (#279)	5,400	0-5 ft. FIII 5-37 ft. Sand & Gravel 37-44 ft. Clay with stones; (TIII?) 44-45 ft. Shale Bedrock @ 45 ft.	Upper Outwash	24,5-29.5	May be suitable upon inspection. Well was constructed with rubber packer and backfilled around screen with native materials.	NS	NS	NS	NS

Hydraulically							Analytical Results		Analytical
Downgradient Wells	Approximate Distance (ft) and Direction from Rosen Site	Geology	Formation Screened	Screen interval (Feet BGL)	Sultability of Well Use	Date	Constituents Detected	Concentration (ug/L)	Method
Elm B (#280)	5,000 NE	0-45 ft. Sand and Gravel 45-50 ft. Fine to coarse Sand, gravel and clay	Upper Outwash	29.5-34.5	May be suitable upon inspection. Well was constructed with rubber packer and backfilled	Sample 2	1,1-dichloroethene	0.2 0.25	Jellers
					around screen with native meterials.	Sample 1 Sample 2	1,1,1-trichloroethane	1.6 0.7	
				,		Sample 2	cis 1,2-dichloroethene	0.1	
						Sample 3 9/90	benzene 1,1,1-trichloroethane	0 4 1.5	
ETL Well A (#281)	4,800 E	0-55 ft. Sand & Gravel 50-60 ft. Clay	Upper Outwash	30-35 and 40-45	East of River.	NS	NS	NS	NS
ETL Well B (#282)	5,000 E	0-55 ft. Sand & Gravel 55-60 ft. Silt/Clay and medium Sand, Clay at 60 ft.	Upper Outwash		East of River.	NS	NS	NS	иѕ
ETL Test Well	5,000 E		No information	No Information	East of River.	NS	NS	NS	NS
Sewage Plant 1	4,000 E-NE	No Information	No Information	20 (open)	May be suitable upon inspection.	NS	NS	NS	NS
Sewage Plant 2	4,000 E-NE	No Information	No information	15 (open)	May be suitable upon inspection.		NS	NS	NS
Brewer Titchener (#181)	6,000 NE	0-47 ft. Sand & Gravel	No Information	No Information	Probable Former Production Well	NS	NS	NS	NS

Hydraulically							Analytical Results		Anglytical
Downgradient Wells	Approximate Distance (ft) and Direction from Rosen Site	Geology	Formation Screened	Screen Interval (Feet BGL)	Suitability of Well Use	Date	Constituents Detected	Concentration (ug/L)	Method
Yaman A		0-10 ft. Gravel and Clay 10-11 ft. Clay 11-13 ft. Coarse Gravel 13-18 ft. Coarse Sand and line Gravel 18-25 ft. Sand and Gravel with Clay 25-27 ft. Medium Sand 27-32.5 ft. Till 32.5-33 ft. Bedrock	Upper Outwash	·	May be suitable upon inspection.		NS		NS
Yaman B	8,000 NE	Depth - 27.5 ft. Geology - same as Yaman A well			May be suitable upon inspection.	4/90	ND		Jeilers

							Analytical Results		Analytical
Hydraulically Upgradient Wells	Approximate Distance (ft) and Direction from Rosen Site	Geology	Formation Screened	Screen Interval (Feet BGL)	Sultability of Well Use	Date	Constituents Detected	Concentration (ug/L)	Method
Dowzer #1	1,000 SW	Depth 23 ft.	No information	No information	May be suitable upon	1/90	1,1,1-trichloroethane	6	EPA 601
(#359)	1,000 011				inspection; however, no information on construction	4/90	ND		Jeffers
				1	(upgradient well).	2/24/92	ND		EPA 502.2
Dowzer #2	400 W	No Information	No information	No information	May be suitable upon inspection; however, no	1/90	1,1,1-trichioroethane	6.6	EPA 601
(#360)			•		information on construction (upgradient well).	4/90 Sample 1	1,1-dichloroethene 1,1,1-trichloroethane	0.5 6.2	Jeflers
						Sample 2	1,1-dichloroethene 1,1,1-trichloroethane 1,4-dichlorobenzene	0.4 4.4 0.5	
Dowzer #2A	400 W	3-7 ft. Silt and Clay 7-23.5 ft. Sand, some grave!	Upper Outwash	9-24	May be suitable upon inspection (upgradient well).	2/24/92	1,1,1-trichloroethane	18 7	EPA 502 2
Dowzer #3	1,000 W	23.5-27 ft. Gravet, sand, some silt	Upper Outwash	9.5-24.5	May be suitable upon inspection (upgradent well).	2/24/92	1,1,1-trichloroethane	10.4	EPA 502 2
High School Bus Garage (#321)	1,000 SE	3-27 ft. Sand and gravel 0-55 ft. Silt with fine to coarse Sand & Gravel 55-75 ft. No Recovery 75-85 ft. Sand & Gravel	Lower .Oulwash	75.5-80.5	May be suitable upon inspection (upgradient well).	9/90	Xylenes Petroleum odor/residue observed 15-17 ft during drilling	0 5	Jeffers
CT-3S	2,000 SW	See CT-3D for Geology		23-28	May be suitable upon inspection.	4/90	ND		Jellers
CT-3D	2.000 SW	5-30 ft. Fill 5-30 ft. Coarse Gravel 30-35 ft. Silty Gravel 35-40 ft. Pebbly Sand 40-80 ft. Sand & Gravel 60-70 ft. Clay & Fine Sand		50-54	May be suitable upon inspection.	4/90	ND		Jeffers

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Hydraulically							Analytical Results		Analytical
	Approximate Distance (ft) and Direction from Rosen Site	Geology	Formation Screened	Screen Interval (Feet BGL)	Sultability of Well Use	Date	· Constituents Detected	Concentration (ug/L)	Method
CT-11	5,000 W	0-15 ft. Clay and Sand	Upper Outwash	38-60	May be suitable upon	4/90	1,1-dichloroethene	0.5	Jeffers
G1-11		15-50 ft. Pebbly Sand 50-80 ft. Sand and Grave!			inspection.	4/90	bromodichloromethane	0.1	
		60-65 ft. Clayey Sand 65-75 ft. Silty Gravel				4/90	1,1,1-trichloroethane	0.5	
ŀ	K.	75-85 ft. Gravel and Sand				9/90		1.4	
		85-100 ft. Pebbly Sand and Clay 100-104 ft. Gray Clay				9/90	trichloroethene	0.04	

Notes:
EPA - Environmental Protection Agency
BGL - Below ground level
ppb - Parts per billion
NS - Not sampled
ND - No constituents detected

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TABLE B-2

CORTLAND COUNTY MONITORING WELL NETWORK SAMPLING RESULTS (4/90 & 9/90)

								<u> </u>								
Well Designation	Date Sampled	1,1-dichloroethene	1,1,1- trichloroethane	1,4- dichlorobenzene	trans 1,2- dichloroethene	ds 1,2- dichloroethene	trichloroethene	tetrachloroethene	bromodichioromethano	methylene Chioride	toluene	xylene	benzene	trichloromethane	carbon tetrachloride	1,1- dichloroethane
CT-45	4/90	****	0.2			0.4	2.7									
0. 10	4/90		0.3			0.4	2.4									
	9/90		1.0		0.2	0.5	2.6									
CT-4D	4/90		0.4			1.1	2.3									
<u> </u>	4/90		0.4			1.0	1.8						 			
	9/90		0.3			0.8	1.9						<u> </u>			
CT-58	4/90					1.8	7.0						 			
-	9/90					10.0	24.2									
CT-5D	4/90					23.8	82.8		ļ				 			
	4/90					27.0	57.0									
	4/90					24.7	77.0						 			
	9/90					16.8	53.9									
CT-11	4/90	0.5	0.5				ļ		01							
	9/90		1.4				0.04						 			
CT-21	4/90					29.2	113.6									
	4/90					34.5	101.4									
	4/90					41.1	121.0									
	9/90		0.2			21.0	66.8									
CT-22	4/90					5.4	12.7									
	9/90		0.5			3.5	14.6	•					 			
CP-14	4/90					0.05	0.26						 			
	4/90					1.5	0.2						 			
	4/90					<0.1	0.2				 					
USGS 89-1	4/90	2.4						 					 			
	4/90	0.1	0.1		1.0			ļ		ļ			 			
	4/90							<u> </u>	<u> </u>		0.8					
	9/90					1.2	0.1			ļ	 		 			
ELMB	4/90	0.2	1.6				<u> </u>	<u> </u>	<u> </u>	L	<u> </u>	L	L	L	L	L

CORTLAND COUNTY MONITORING WELL NETWORK SAMPLING RESULTS (4/90 & 9/90)

						1		1						. 1	
Date ampled	1,1-dichloroethene	1,1,1- trichloroethane	1,4- dichlorobenzene	trans 1,2- dichloroethene	cls 1,2- dichloroethene	trichloroethene	tetrachloroethene	bromedichloromethane	methylene Chloride	toluene	xylene	benzene	trichloromethane	carbon tetrachloride	1,1- dichloroethane
4/90	0.25	0.7			0.1							0.4			
4/90															
9/90		1.5										 			
4/90	0.5	6.2					ļ								
4/90	0.4	4.4	0.5				ļ								
4/90	1.1	11.5													
4/90	30	15.0				ļ				trace		14			
4/90						 									5.2
9/90		13 8		0.6						0.4					
4/90						0.06	 				09				
9/90		0.02		0.2			0.3								
9/90		5.2										-			
9/90		6.1			6.8	17.0	0.4					ļ	 		
9/90		0.04		0.2							 -	-			25 0
9/90		107.5		28.9		}			 -	 					
9/90		2.7					ļ		 	 	 -				
9/90		1.4		0.3				 							
9/90		0.35					 	 		 				l	
9/90							 								<u></u>
9/90		0.4		0.3			 	- 	 	 	 			L	
9/90		2.4			2.1				 	 				1	1
4/90	0.2	0.4				0.03						ļ		ļ	
		0.5			tace	trace		<u> </u>			<u> </u>	 	 	 	
	trace				0.03	0.04				ļ	<u> </u>	<u> </u>	 	 	
9/90						0.95				<u> </u>	<u> </u>	<u> </u>		<u> </u>	J
	Argo 4/90 4/90 9/90 9/90 4/90 9/90 4/90 9/90 4/90 9/90 4/90 9/90 4/90 9/90 4/90 9/90 4/90 9/90 4/90 9/90 4/90 9/90 9	Ampled 1,1-dichloroethere 4/90 0.25 4/90 0.5 4/90 0.5 4/90 0.4 4/90 1.1 4/90 3.0 4/90 9/90 9/90 9/90 9/90 9/90 9/90 9/90 9	### 1-dichloroethene trichloroethene #### 14/90 ### 1.1-dichloroethene ### 15/90 ### 1.5 ### 1	1,1-dichloroethene trichloroethane dichlorobergene	1,1-dichloroethene	Delis (1,1-dichicroethene tichicroethene dichicroberuzene dichicroethene dichicro	Dela	Dela ministration Dela	Delay	Date 1.1-f-chioroethere 1.1.1	Dale 1.1-dishlorothere 1.1.1-	Delay Dela	Della Marie Mari	Delay 1,1-1- 1,1-1- 2,	Date winder 1,1,1-1- fest part 1,2 fest part 1,1,1- fest part 1,2 fest part 1,1,1- fest par

CORTLAND COUNTY MONITORING WELL NETWORK SAMPLING RESULTS (4/90 & 9/90)

	<u> </u>								·	methylene					carbon	1,1-
Well Designation	Date Sampled	1,1-dichloroethene	1,1,1- trichloroethane	1,4- dichlorobenzene	trans 1,2- dichloroethene	cis 1,2- dichlorcethene	trichloroethene	tetrachloroethene	bromodichloromethane	Chloride	toluene	xylene	benzene	trichloromethane	tetrachionde	dichloroethane
	4/90	trace	1.1			0.4	0.9									
MW-3S	4/90		2.3			0.65	0.03									
	4/90		2.2			0.7	0.04									
dup.	4/90		3.3			0.22	0.06	, 0.02								
dup.	4/90		1.8			0.7	trace									
dup.	4/90		1.2			0.2										
dup.	4/90		2.5			0.3				0.2						
	9/90		0.8			0.3	0.9									
MW-3D (MW3CiTYWW)	4/90		6.5			0.4										
	4/90	0.6	6.1												ļ	
	9/90		3.2			0.3	0.06							ļ		
PW3CORTWW	4/90		0.25													
PW4CORTWW	4/90		1.2			0.2	0.02									····
	4/90	0.1	0.8			0.15	0.11									
	9/90		0.6													
⊺-103	4/90		0.15						trace	<2						
	9/90				0.3	0.4	0.2								<u> </u>	
Jebbett	4/90					0.04	1.2							ļ		
2. Stupke	4/90					6.9	50.3	•						 	 	
	9/90					2.3	19.5							ļ		
	9/90					3.1	25.3		·					<u> </u>	 	
ACE.	4/90					62.2	100.8							ļ	 	
	4/90	trace			trace	74.7	88.0								 -	
	9/90		0.2			32.2	37.6									
	9/90					35.0	53.0									
	9/90		0.15			36.0	59.0								ļ	<u> </u>
ITS	4/90					0.54	15 6						L	l	<u> </u>	L

CORTLAND COUNTY MONITORING WELL NETWORK SAMPLING RESULTS (4/90 & 9/90)

CORTLAND, NEW YORK

														1		i .	A .
			•										l .		carbon	1,1-	1
										metrylene		xylene	benzene	trichloromethane	tetrachloride	dichloroethane	4
				4.4	trans 1,2-	cis 1,2·	trichloroethene	tetrachloroethene	bromodichloromethane	Chloride	toluene	Ayerre	-				1
Wel	Date		1,1,1-	1,4- dichlorobenzene	m 44	dichloroethene	AKSACTORUCIO						<u> </u>	ļ			1
Designation	Sampled	1,1-dichloroethene	trichloroethane				0.7	0.75					l		 		1
	4/90		0.2			0.1	0.5	0.4			 					 	1
53 Green Bush			0.3	Ĭ			0.8	0.5					1	0.7	<0.2		1
	4/90		0.4			ļ		0.25		ļ	bace	ŀ	1			 	1
	9/90	ļ			0.3	1					4.5	2.0	trace		<u> </u>	ļ	-
Cortland Co.	4/90	•			<u></u>	ļ	 					5.3	2.1				-
Office	L		0.5	1		<u> </u>	 			<u> </u>	42.5		+	 			_]
Old Hess	4/90	0.3			0.4	0.06	0.1	 	<u> </u>			0.5			1		_
	9/90	l	0.3	 													
HSBUSGAR	9/90		i	<u></u>	<u> </u>												
ILUSO COOK		<u> </u>															

Only wells in which detected constituents were present are listed. Concentrations are in ug/L equivalent to parts per billion (ppb), dup, - indicates duplicate cample

CORTLAND COUNTY MONITORING WELL NETWORK SAMPLING RESULTS (4/90 & 9/90)

				, , , , , , , , , , , , , , , , , , ,			<u> </u>			İ					carbon	1,1-
Well	Date Serrolari	1,1-dichloroethene	1,1,1- trichloroethene	1,4- dichloroberizene	trans 1,2- dichloroethene	cis 1,2- dichloroethene	trichloroethene	tetrachloroethene	bromodichloromethane	methylene Chloride	toluene	xylene	benzene	trichloromethane		
Designation	-		0.05			0.4	9.5						ļ			
	9/90		0.03			0.3	10.4							 		
	9/90					0.8	11.4							 		
	9/90					18.2	45.9		<u> </u>				 			
PTM	4/90		0.2			10.4	25.3	<u></u>			ļ		 	 		
	9/90		0.5			11.0	30.6				 					
	9/90		3.1			14.9	49.1	2.5					ļ			
OVDDOOR	4/90	0.4	5.8			15.8	39.3	2.2								
	4/90	0.4	3.1			13.5	46 4	2.8			<u> </u>			 	ļ	
	4/90		6.5			12.5	43.2	3.1		ļ		ļ	 	 		
	9/90		6.3			10.9	37.4	2.0					 	03	 	
	9/90		6.3			17.3	53.8		<u> </u>		ļ		 	03	 	
SUBST	4/90					23.0	47.2	0.3				ļ			ļ	
	4/90	0.15				15.4.	37.3				<u> </u>	ļ	ļ	 		
	9/90		0.2			15.4	49.6						ļ	 		
	9/90		0.4			13.0	71.0			ļ			ļ	<u> </u>		
SPCA	4/90			 		12.6	65.1		<u> </u>		ļ		ļ	<u> </u>		
	4/90			 		13.2	44.9			<u>'</u>			 	 	ļ	
	4/90		201			6.9	30.5				ļ	ļ	<u> </u>	 	ļ	
	9/90		0.01			14.6	52.5				<u> </u>		ļ	 	 	
	9/90		0.04			16.0	62.8				 		 	 	 	
	9/90		0.04	 		T				<u> </u>	0.8	trace			 	
Mobil	4/90	0.3	3.2	 		8.5	17.2				 	 	 	 	 	
Pauldine	4/90		4.3	 		11.6	17.8			<u> </u>	 	<u> </u>	 	 	 	
	4/90	1.1		 		5.1	6.9			<u> </u>	<u> </u>	L		 		
	9/90		1.2	 			1				0.1	ļ	 	ļ	 	
Petrello	4/90					0.14	<0.1				1				1	
Contandville	4/90			1		1	<u> </u>	<u> </u>	<u> </u>	L	1	L	1			<u> </u>

